

Dynamic design of track transition between two different slab tracks

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With the development of high-speed rail system, various types of slab track were put into service in Europe, China and Japan.



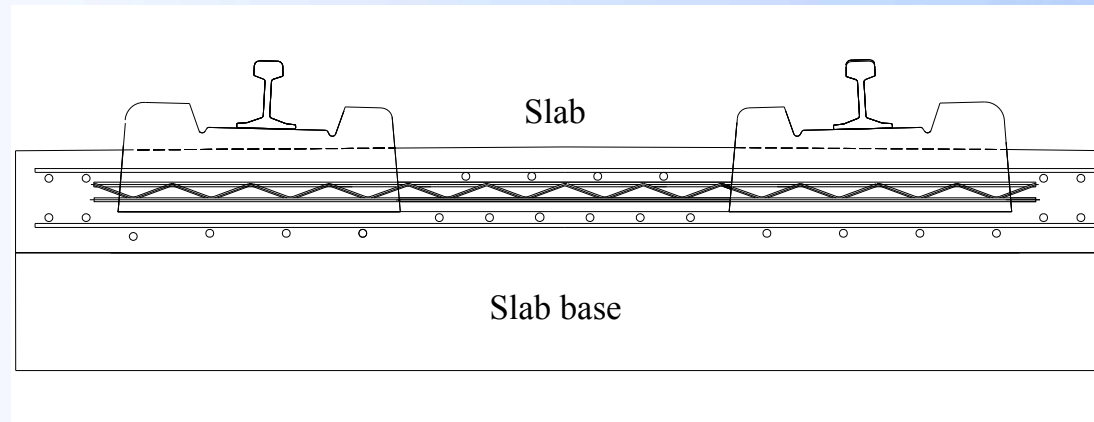
China Railway Track System (CRTS)

Generally speaking, it has been substantiated that the noise level on slab track was 5dB higher than that on the ballast track. (Quarterly report of RTRI, 1997)

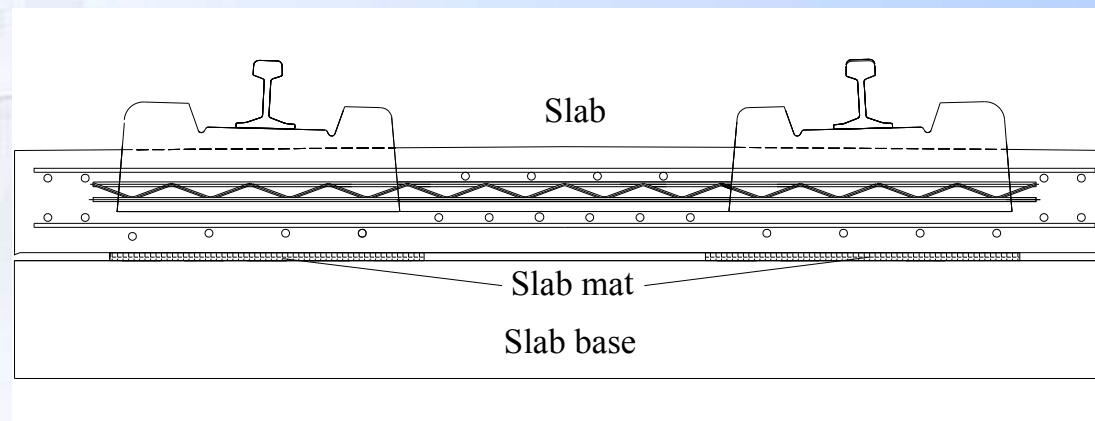
Rail vibration levels on the slab track are 5db higher than those on the ballasted track, particularly for frequencies above 1kHz. (Wang 2010)



In order to reduce track vibration into surrounding structures, new slab track is specially designed for vibration sensitive areas like railway stations by inserting soft rubber mats under slabs. The newly designed track is referred to as floating slab track.



Fixed slab track



Floating slab track

At the transition point between fixed slab track and floating slab track, a moving wheel experiences a rapid change in elevation and dynamic problems occur because of the abrupt change in the vertical track stiffness.

Transition regions require frequent maintenance. When neglected, the track geometry will deteriorate at an accelerated rate. (Lei & Zhang, 2011)

A vehicle-track dynamic analysis model is established based on self-developed simulation package FORSYS to study the dynamic behaviours of track transition between fixed slab track and floating slab track.

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The vehicle-track system model is composed of vehicle model, track model and wheel/rail interaction model.

Vehicle model

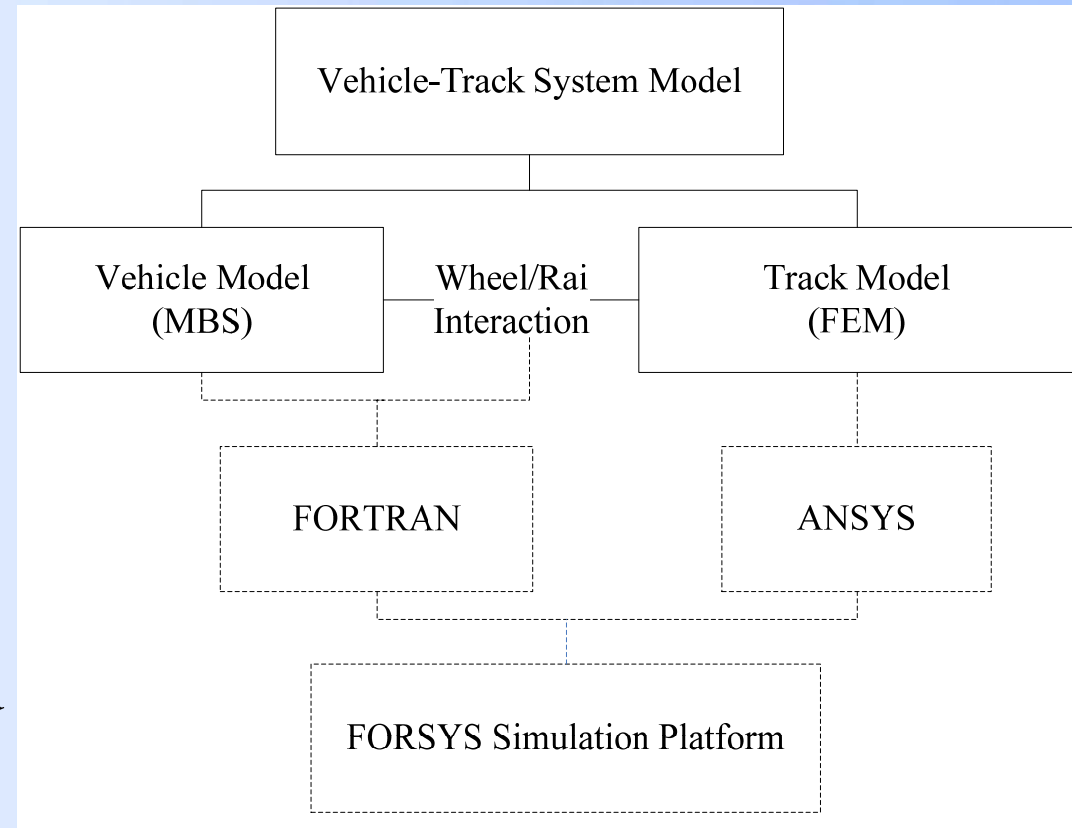
- MBS
- FORTRAN code

Track model

- FEM
- ANSYS software

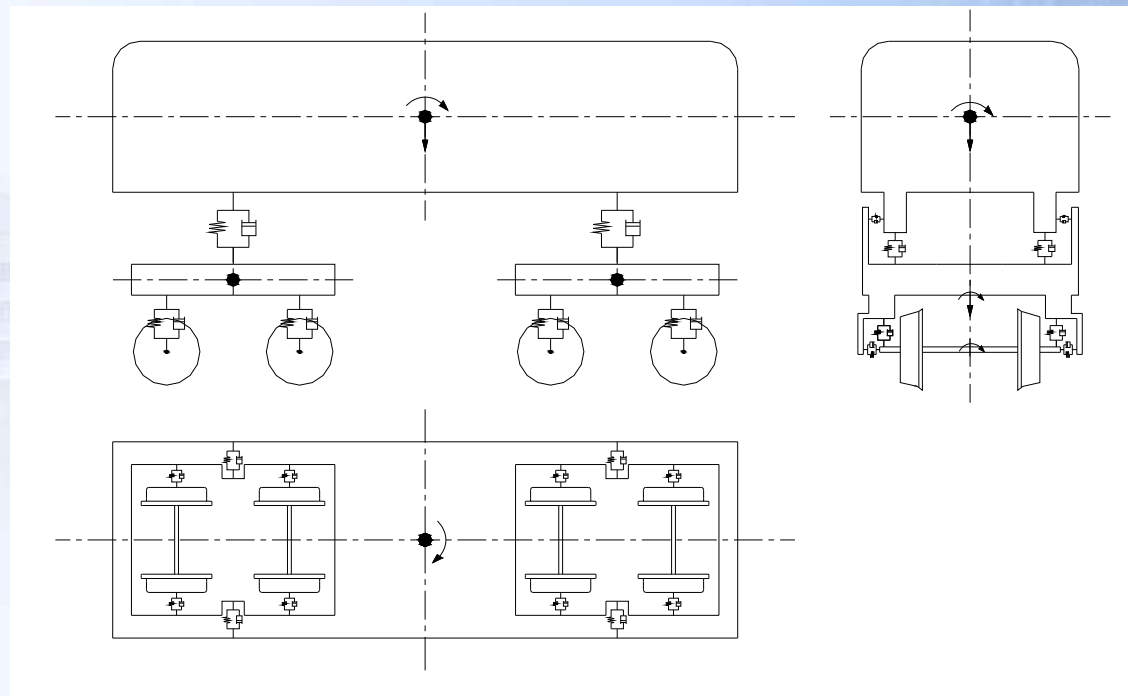
WRI model

- rigid-flexible coupled
- FORSYS platform



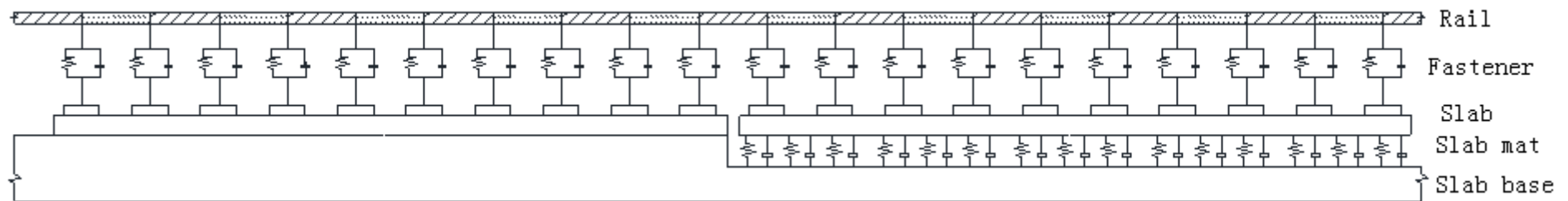
The vehicle is composed of car body, bogies and wheel-sets.

- Car body, bogies and wheel/sets
 - Rigid bodies
- Primary and secondary suspensions
 - spring-dampers



The slab track is composed of rail, fastener, slab and slab mat.

- Rail
 - point supporting beam element
- Fastener
 - spring-damping element
- Slab
 - solid element
- Slab mat
 - spring-damping element



The wheel/rail contact force is calculated by Hertz nonlinear elastic contact theory as following:

$$P = \left(\frac{\Delta Z}{G} \right)^{3/2}$$

Where G is contact coefficient, ΔZ is elastic penetration between wheel and rail. The penetration is determined by the relative displacement of wheel and rail at the wheel/rail contact point.

$$\Delta Z = Z_w - Z_r - R$$

Where Z_w is the vertical displacement of wheel, Z_r is the vertical displacement of rail and R refers to the irregularities of rail surface.

The system vibration differential equation can be expressed as following:

$$\begin{bmatrix} M_m & 0 \\ 0 & M_f \end{bmatrix} \begin{Bmatrix} \ddot{\delta}_m \\ \ddot{\delta}_f \end{Bmatrix} + \begin{bmatrix} C_m & 0 \\ 0 & C_f \end{bmatrix} \begin{Bmatrix} \dot{\delta}_m \\ \dot{\delta}_f \end{Bmatrix} + \begin{bmatrix} K_m & 0 \\ 0 & K_f \end{bmatrix} \begin{Bmatrix} \delta_m \\ \delta_f \end{Bmatrix} = \begin{Bmatrix} P_m \\ P_f \end{Bmatrix}$$

The subscripts m and f represent MBS (vehicle model) and FEM (track model).

After the above system vibration equations obtained, Newmark- β method is applied to solve the equations in the time domain.

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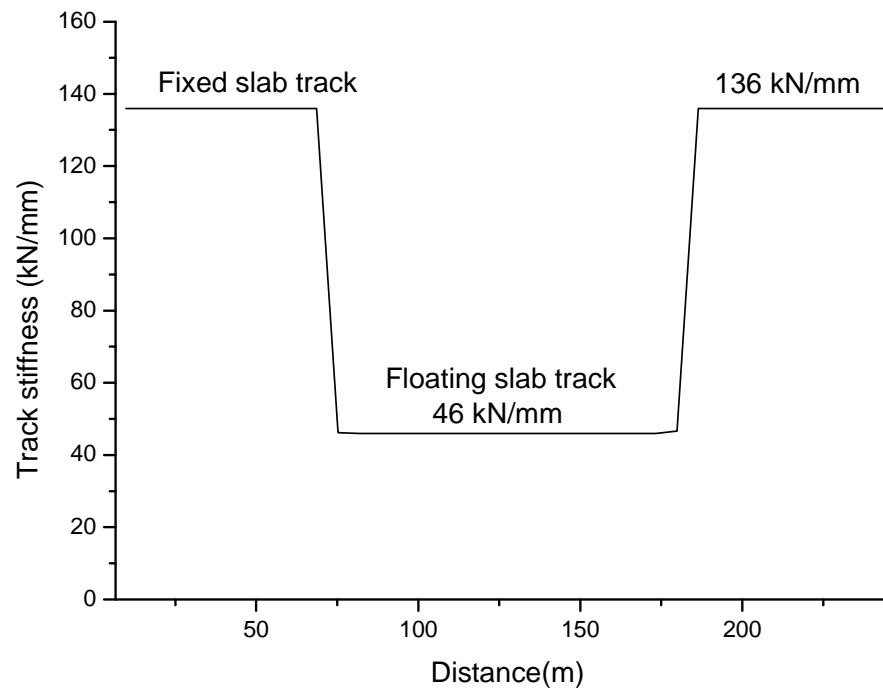
At the transition point between fixed slab track and floating slab track, a moving wheel experiences a **rapid change in elevation** and **dynamic problems** occur because of the **abrupt change in the vertical track stiffness**.



- Track stiffness/modulus
- Wheel/rail contact force
- Rail deflection

Track stiffness (K_t) is the ratio of the applied wheel load (P) to rail deflection (y):

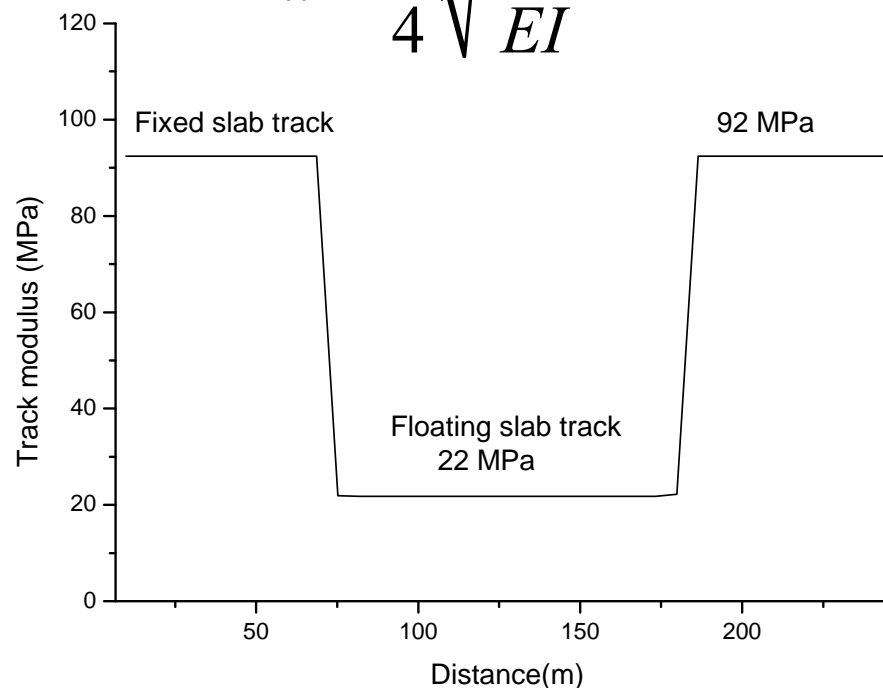
$$K_t = \frac{P}{y}$$



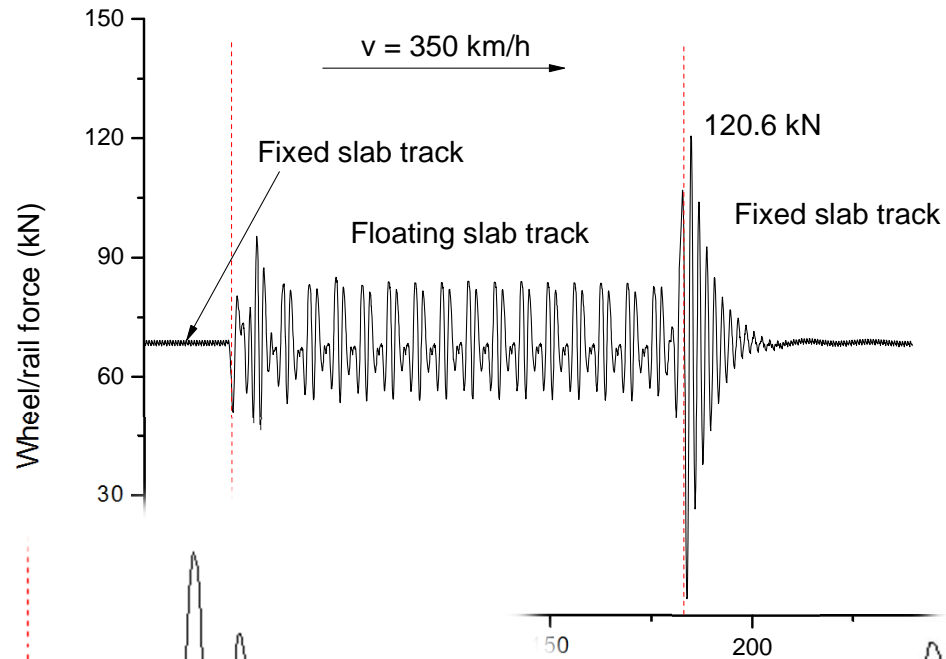
$$136/36 \approx 3$$

Track modulus is often used as a measure of vertical stiffness of the rail foundation and is defined as the supporting force per unit length of rail per unit vertical deflection under a vertical load, as determined by the following equation (Selig and Waters 1994):

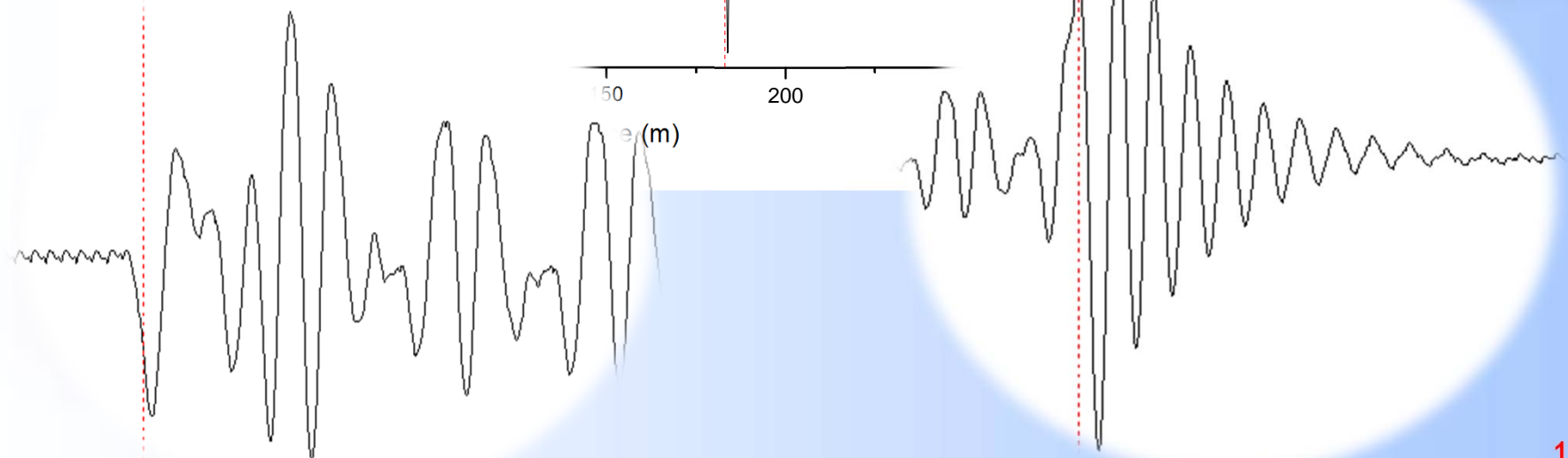
$$u = \frac{1}{4} \sqrt[3]{\frac{K_t^4}{EI}}$$

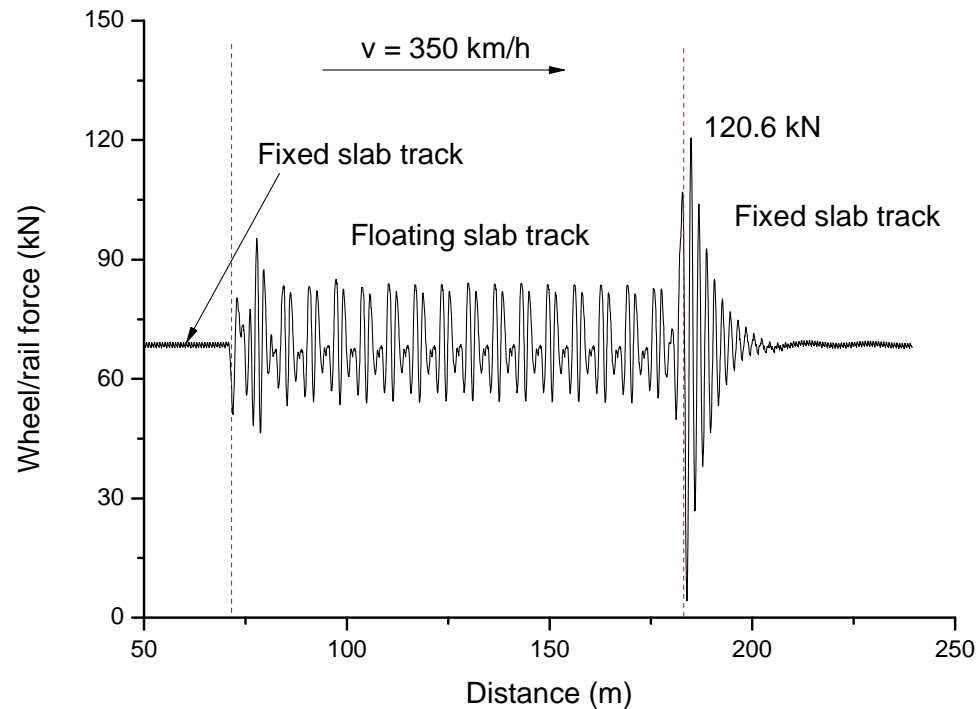


$$92/22 \approx 4$$



Dynamic amplification factor:
 $F_d / F_0 = 120.6 / 68.6 \approx 1.76$

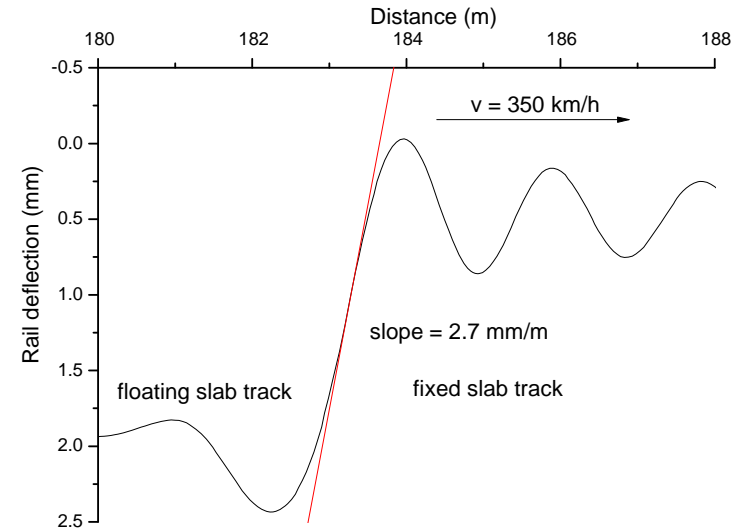
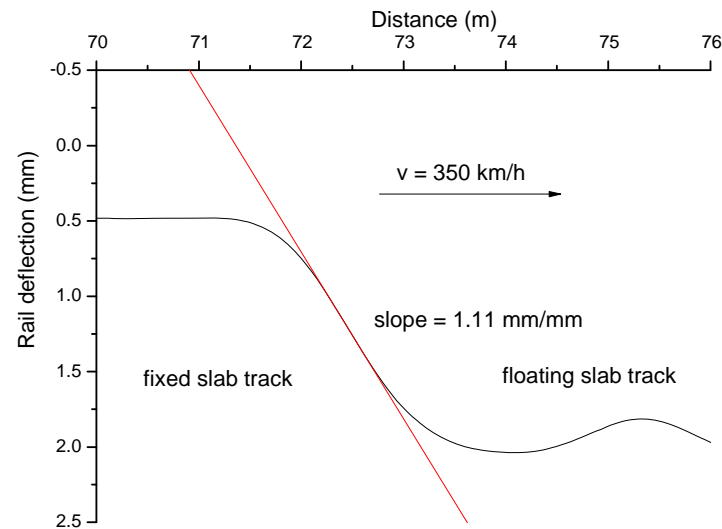




Dynamic amplification factor:
 $F_d / F_0 = 120.6 / 68.6 \approx 1.76$

The dynamic analysis results show that the wheel/rail interaction is larger when vehicle passes the transition from low-stiffness side to high-stiffness side, compared to the passing in the opposite direction.

Even without any initial track irregularities, an abrupt track stiffness change alone would lead to 76% higher dynamic load than static load in the transition.



The rail deflection difference is 1.5mm occurring in about 2m.

The maximum variation rate of rail deflection is 2.7mm/m, far more than expected.

At the transition point between fixed slab track and floating slab track, a moving wheel experiences a **rapid change in elevation** and **dynamic problems** occur because of the **abrupt change in the vertical track stiffness**.



- Track stiffness/modulus: 3, 4 times
- wheel/rail contact force: 68.6→120.6kN, 1.76
- Rail deflection: 1.5mm/2m, 2.7mm/m

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Increase stiffness of low-stiffness side

- Additional rail
- Long/wide sleeper
- Reducing sleeper spacing
- Approach slab
- Glued ballast

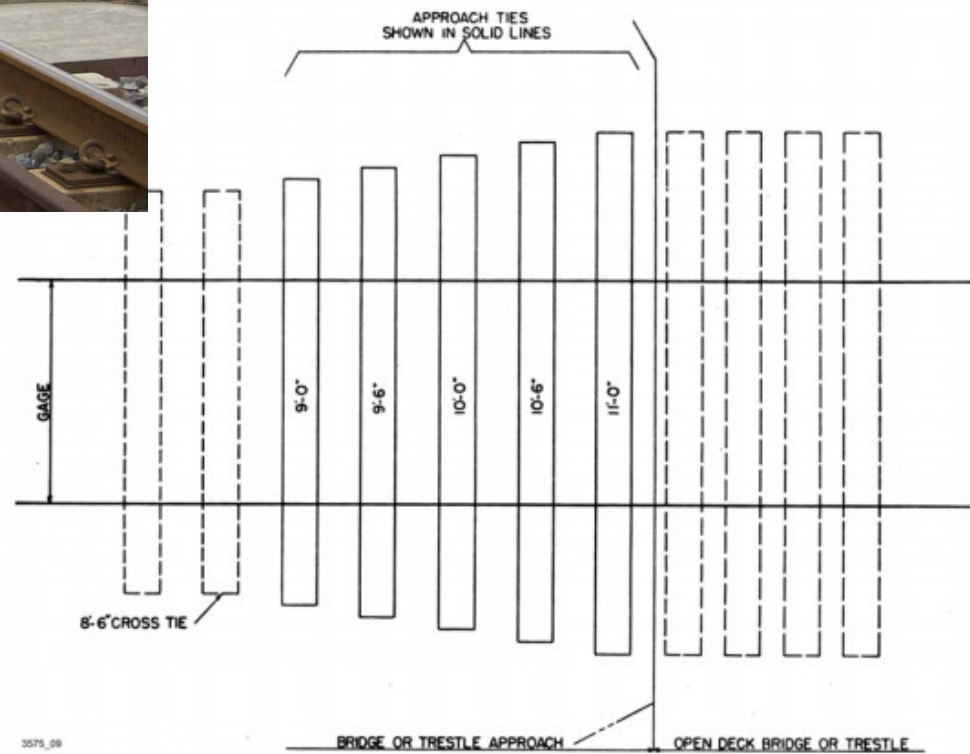
And/or

Decrease stiffness of high-stiffness side

- Rail seat pad
- Sleeper pad
- Slab mat
- Ballast mat

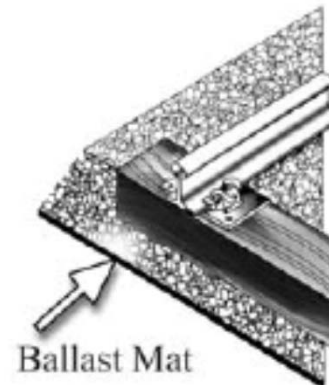


Additional rail



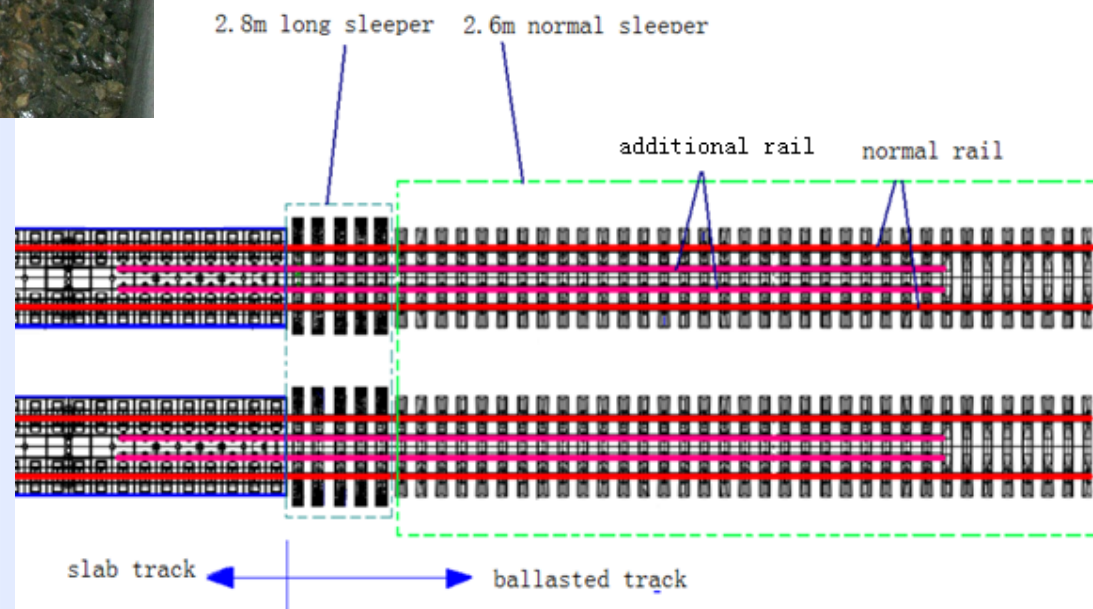
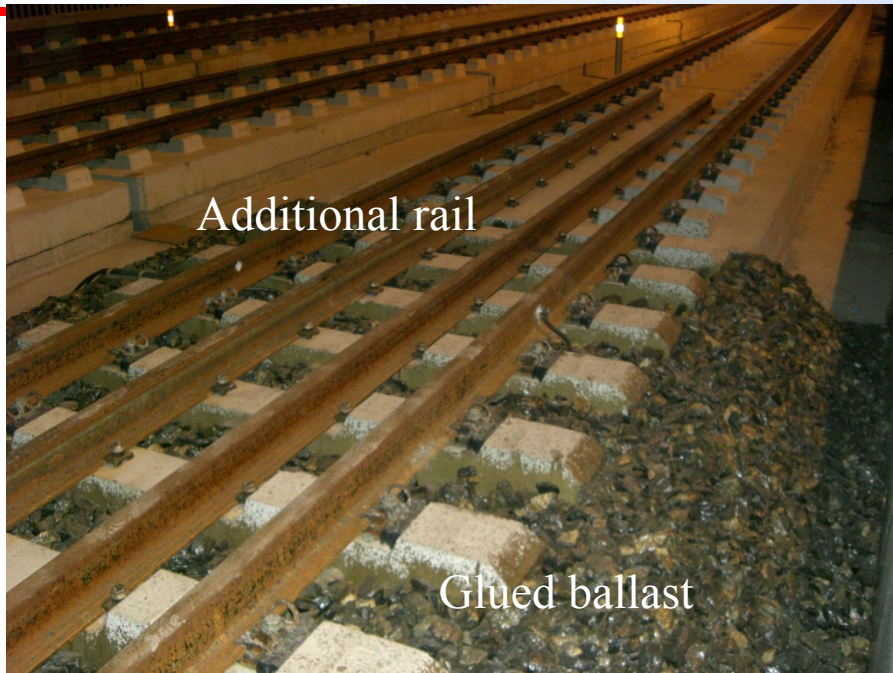
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Source: AREMA 2005a.



(Sasaoka & Davis 2005)

Transition Remedies



Increase stiffness of low-stiffness side

- **Additional rail**
- Long/wide sleeper
- Reducing sleeper spacing
- Approach slab
- Glued ballast

And/or

Decrease stiffness of high-stiffness side

- **Rail seat pad**
- Sleeper pad
- **Slab mat** ✓
- Ballast mat

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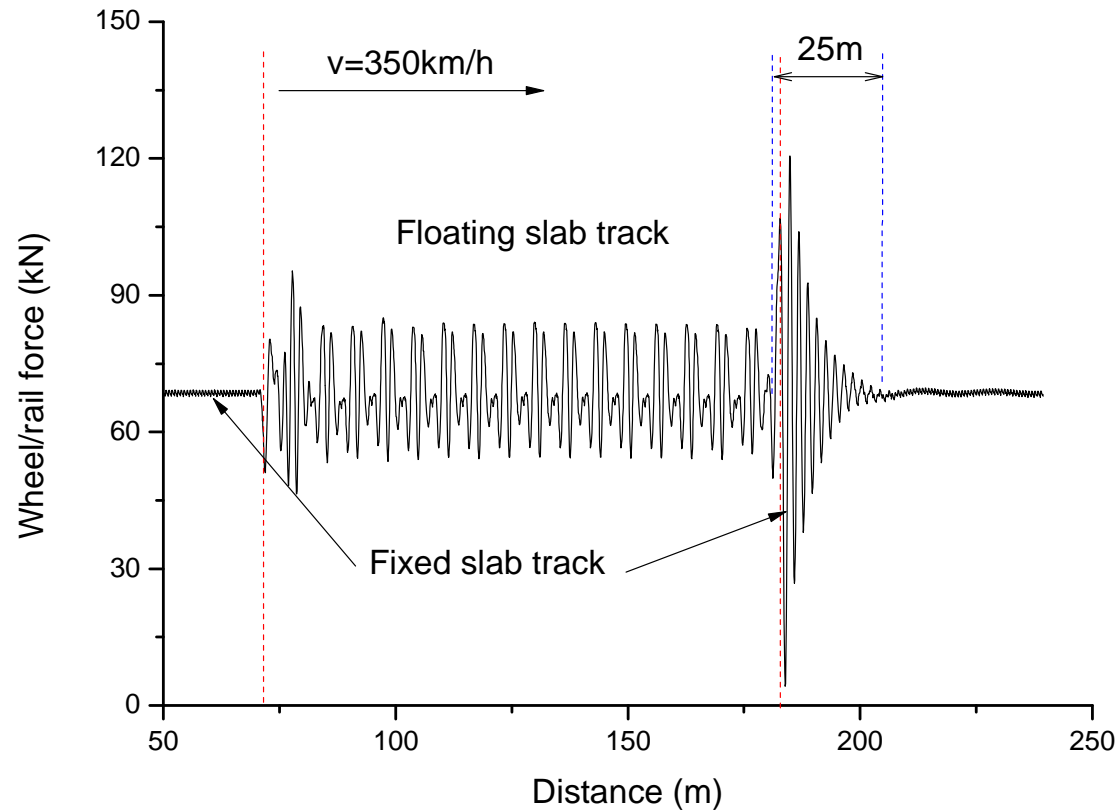
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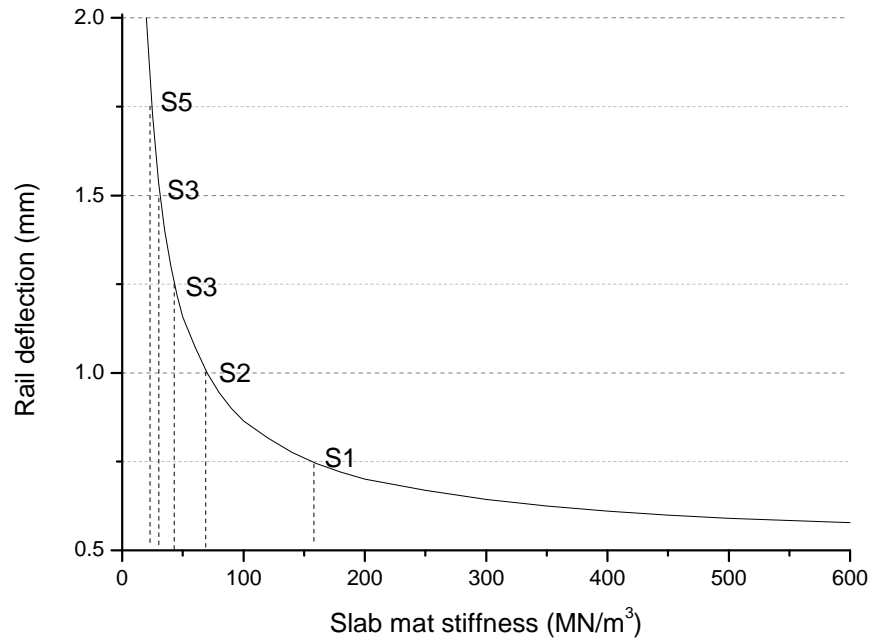
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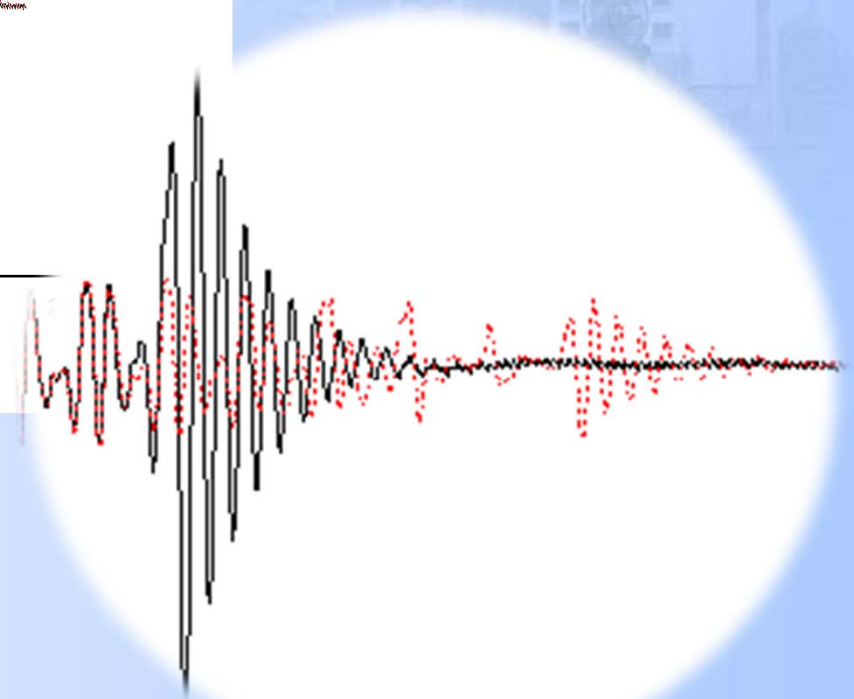
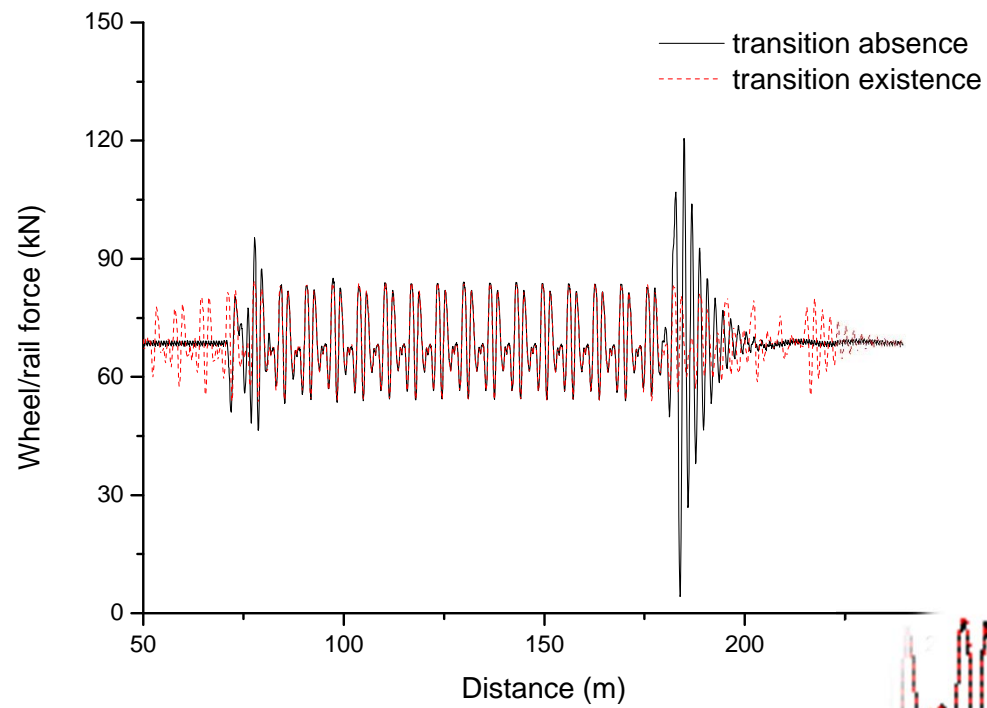


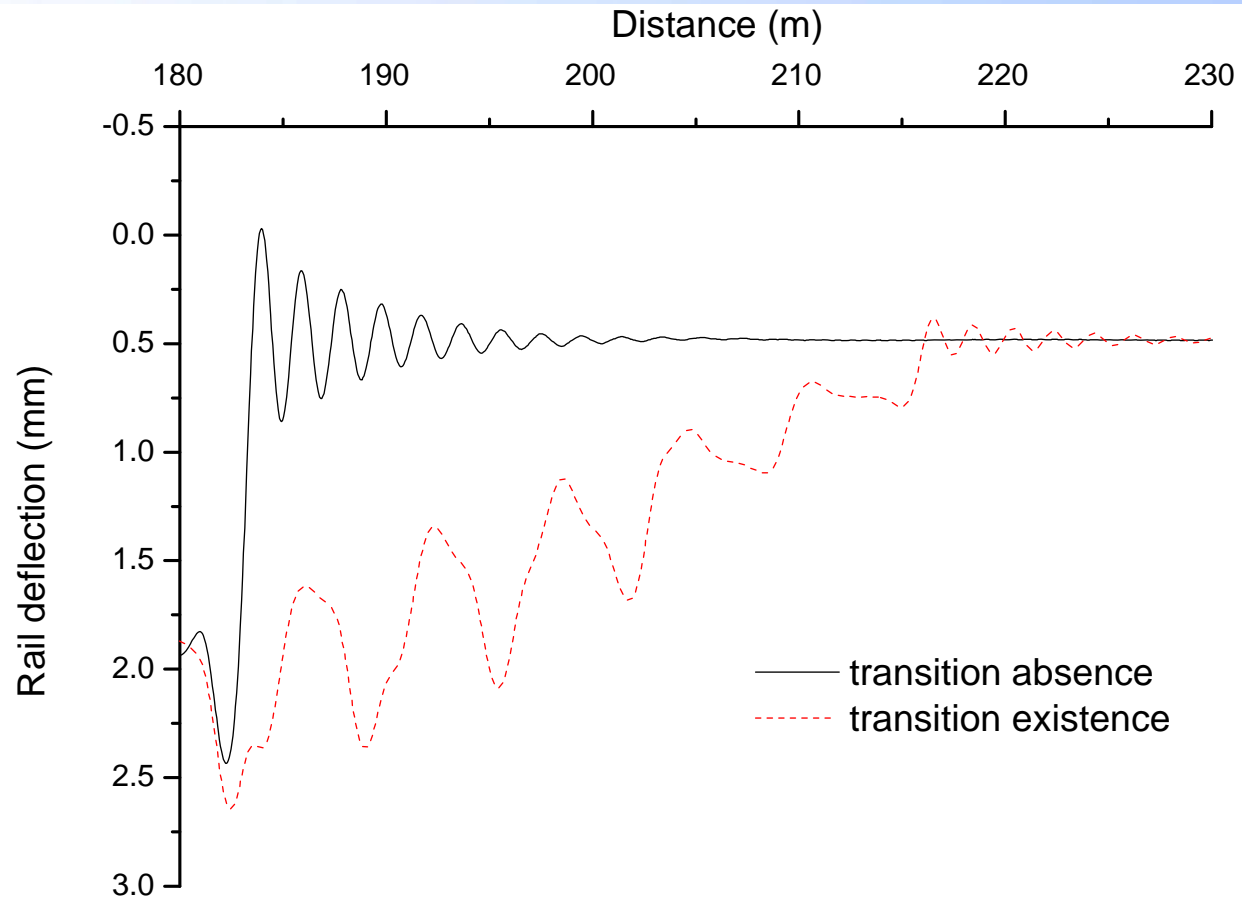
Transition length: $25\text{m} < L < 50\text{m}$, $6.54\text{m} \times 5 = 32.7\text{m} \checkmark$



- Equalize the rail deflection
- Provide a gradual stiffness increase

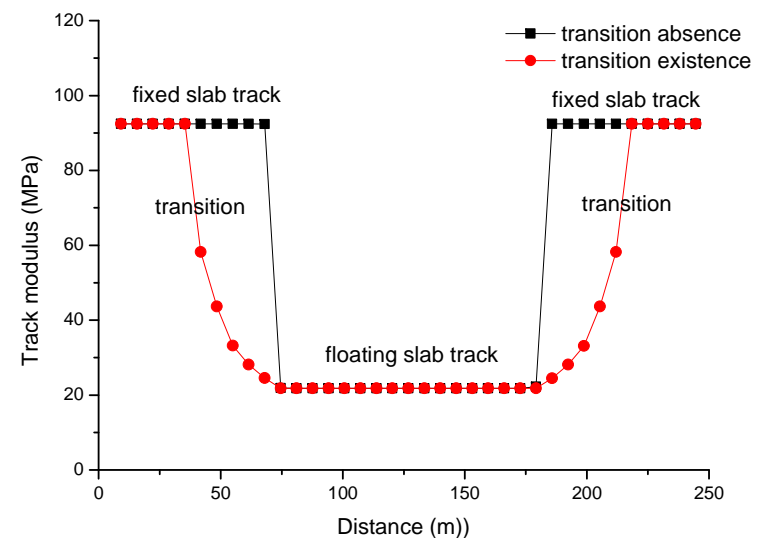
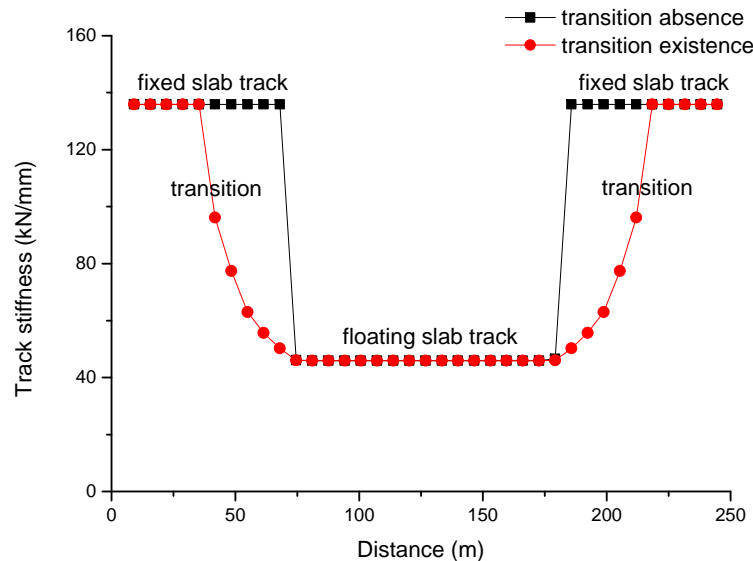
Railway track		Rail deflection(mm)	Slab mat stiffness(MN/m ³)
Fixed slab track		0.5	-
Track transition	S1	0.75	150
	S2	1.0	70
	S3	1.25	40
	S4	1.5	30
	S5	1.75	24
Floating slab track		2.0	20





Railway track		Slab mat stiffness (MN/m ³)	Track stiffness (MN/m)	Track modulus (MPa)
Fixed slab track		-	136	92
Track transition	1	150	96	58
	2	70	77	44
	3	40	63	33
	4	30	56	28
	5	24	50	24
Floating slab track		20	46	22

A 'gradual' increase in track stiffness and modulus dose not mean linear change.



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- Track faults
 - ✓ Rail surface irregularities
 - ✓ Long-term track deformation
- Integrated transition remedies
 - ✓ Slab mat
 - ✓ Additional rail
 - ✓ Fastener stiffness
 - ✓ ...

Thank you for your kind attention!

