



# AF2903 Road Construction and Maintenance

## Design of Asphalt Mixtures

Royal Institute of Technology  
Stockholm, April 8<sup>th</sup> 2014

Dr. Alvaro Guarin

Highway and Railway Engineering  
Department of Transportation Science





# Hot Mix Asphalt Design

Objective:

Develop an economical blend of aggregates and asphalt that meet design requirements

Most important mix design methods

- Marshall
- Superpave



# Requirements in Common

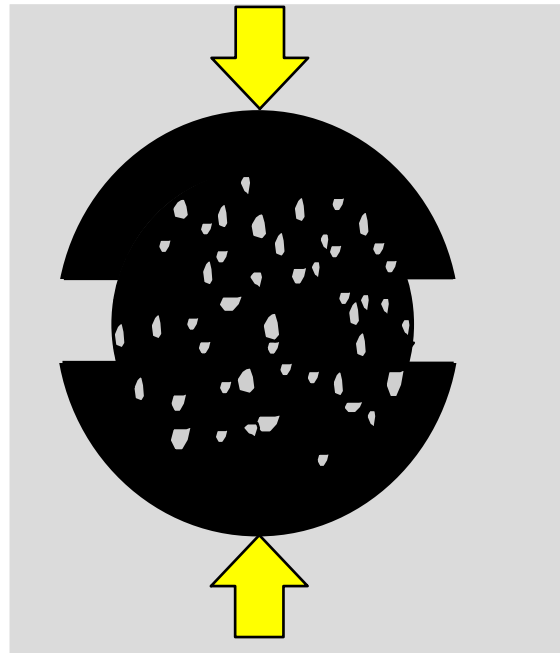
- Sufficient asphalt to ensure durability
- Sufficient stability under traffic loads
- Sufficient air voids

Lower limit to allow room for initial densification due to traffic (bleeding)

Upper limit to prevent excessive environmental damage (aging)

- Sufficient workability

# MARSHALL MIX DESIGN



Developed by Bruce Marshall for the Mississippi Highway Department in the late 30's

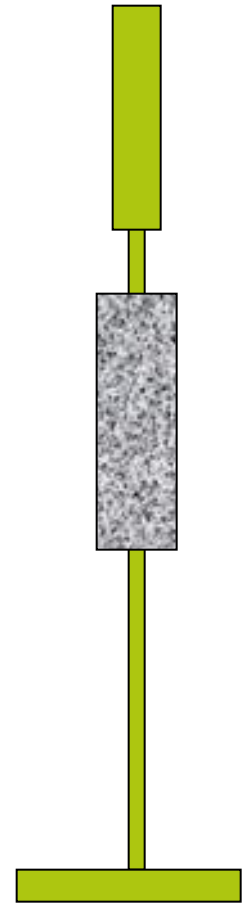
Evaluated compaction effort

Hammer weight: 10 lb

50 blows/side as an initial standard

4% voids after traffic

Initial criteria were established and upgraded for increased tire pressures and loads



# Marshall Hammer





# Marshall Mix Design

Select and test aggregate

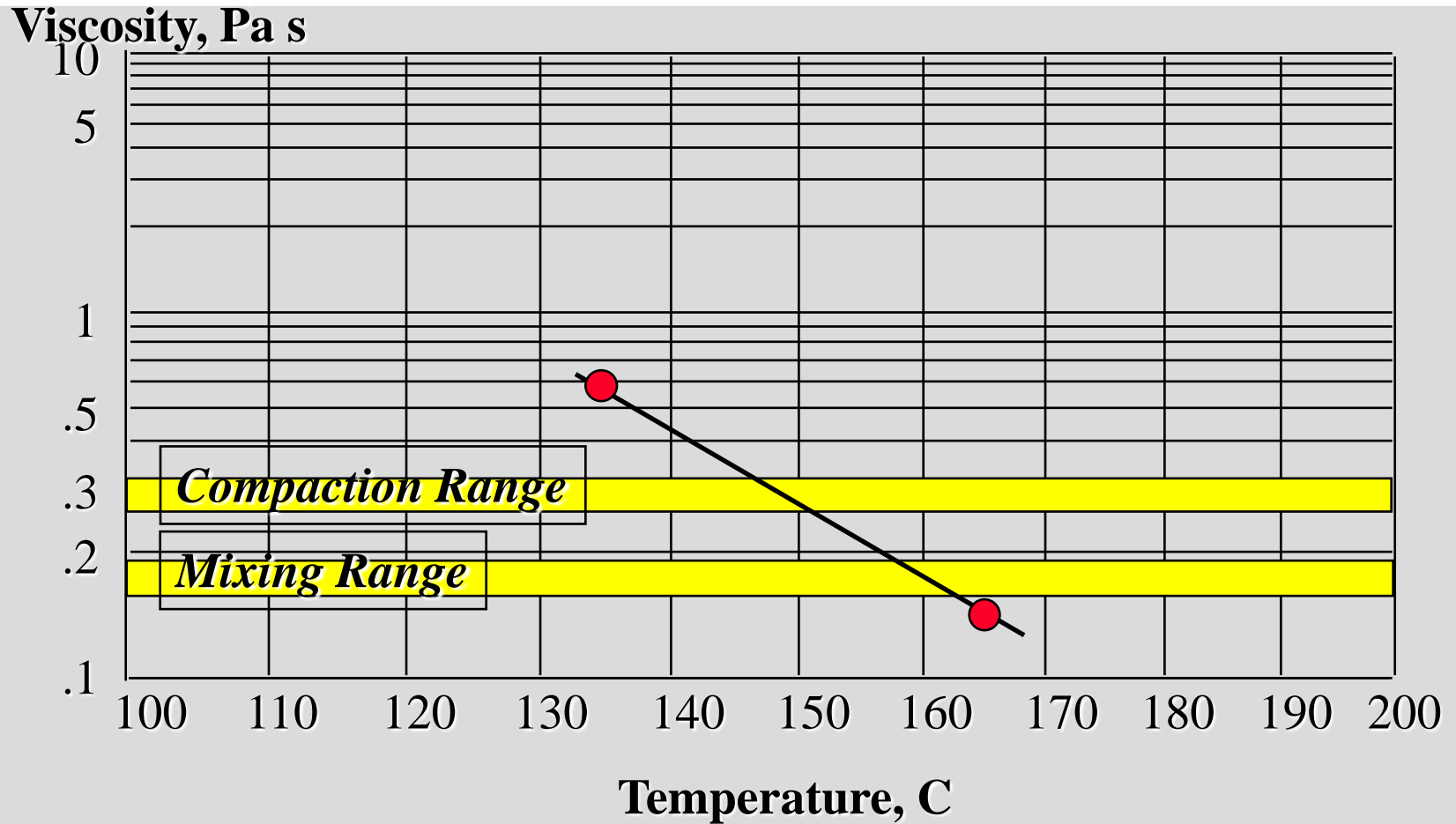
Select and test asphalt cement

Establish mixing and compaction temperatures

Develop trial blends

Heat and mix asphalt cement and aggregates  
Compact specimen (100 mm diameter)

# Mixing/Compaction Temperatures







# Marshall Design Criteria

	<b>Light Traffic ESAL &lt; 10<sup>4</sup></b>	<b>Medium Traffic 10<sup>4</sup> &lt; ESAL &lt; 10<sup>5</sup></b>	<b>Heavy Traffic ESAL &gt; 10<sup>6</sup></b>
--	---	--	---

<b>Compaction</b>	<b>35</b>	<b>50</b>	<b>75</b>
<b>Stability N (lb.)</b>	<b>3336 (750)</b>	<b>5338 (1200)</b>	<b>8006 (1800)</b>
<b>Flow, 0.25 mm (0.1 in)</b>	<b>8 to 18</b>	<b>8 to 16</b>	<b>8 to 14</b>
<b>Air Voids, %</b>	<b>3 to 5</b>	<b>3 to 5</b>	<b>3 to 5</b>
<b>Voids in Mineral Agg. (VMA)</b>	<b>Varies with aggregate size</b>		



# Marshall Mix Design Tests

Bulk specific gravity of compacted sample

Maximum specific gravity of loose mix

Stability and flow

60°C water bath (30 to 40 minutes)

50 mm/min loading rate

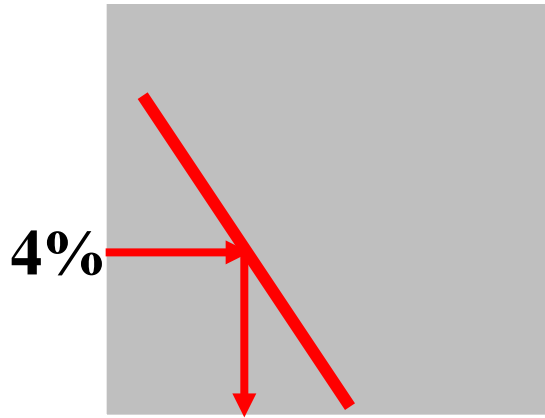
Max. load = uncorrected stability

Corresponding vertical deformation = flow

# Marshall Stability and Flow

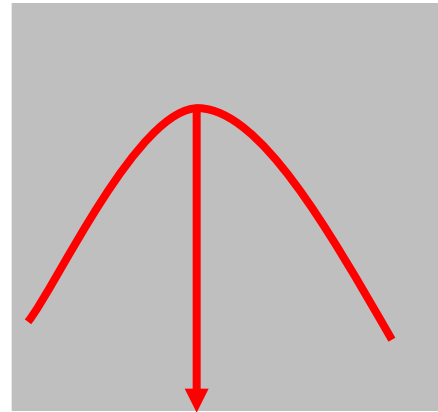


**Air Voids, %**



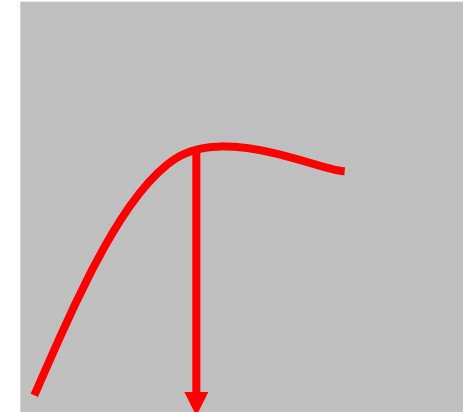
**Asphalt Content, %**

**Stability**



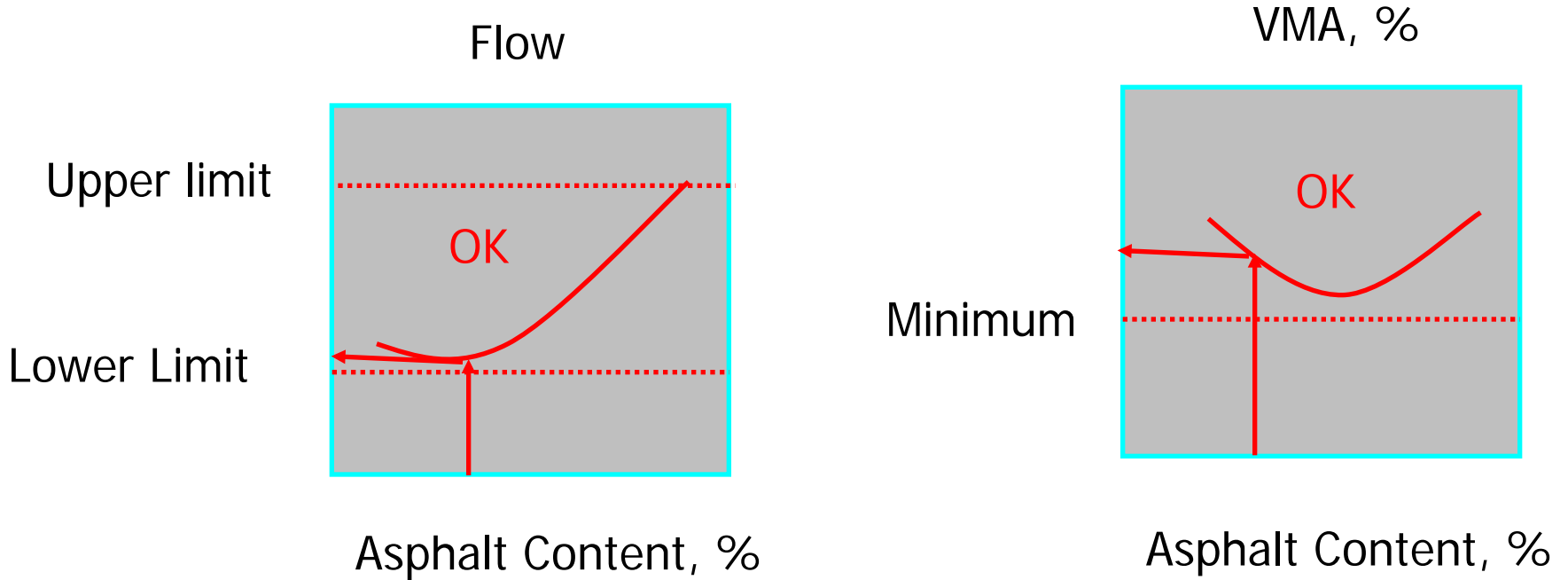
**Asphalt Content, %**

**Gmb**



**Asphalt Content, %**

***Target optimum asphalt content = average***



*Use target optimum asphalt content to check if these criteria are met*

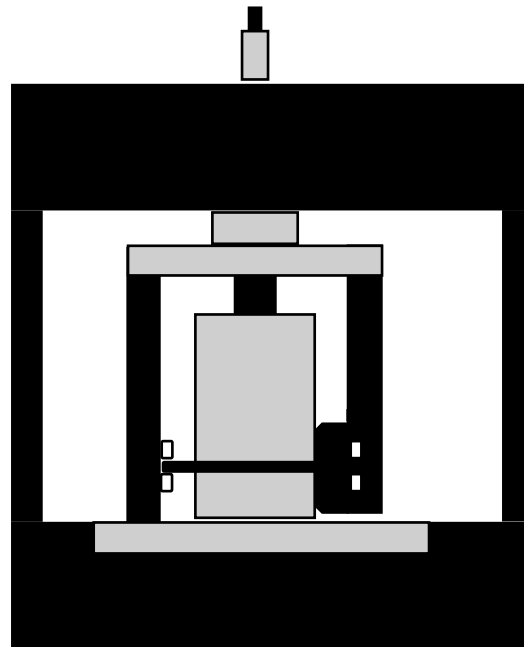
## Advantages

- Attention on voids, strength, durability
- Inexpensive equipment
- Easy to use in process control/acceptance

## Disadvantages

- Impact method of compaction
- Does not consider shear strength
- Load perpendicular to compaction axis

# Superior Performing Asphalt Pavements





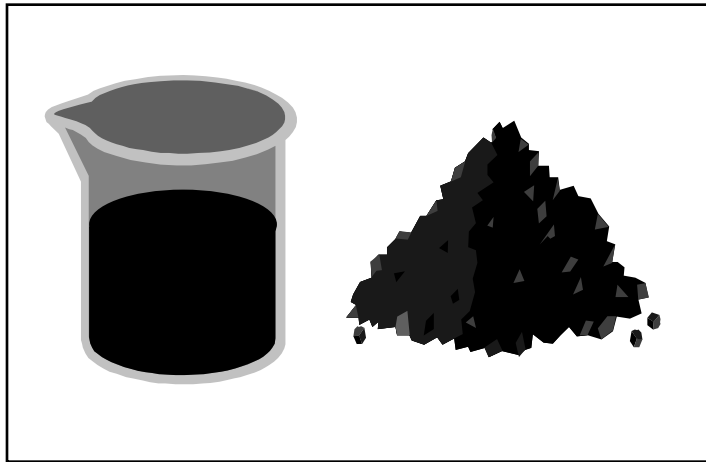
# Gyratory Compaction and Mixture Requirements

Section objectives:

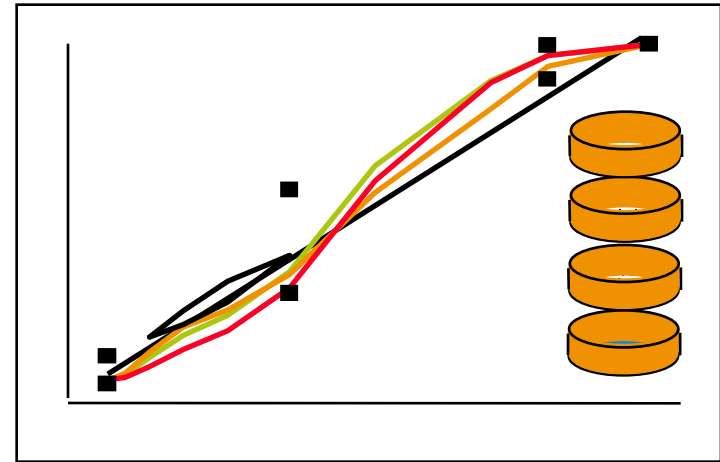
- Describe the Superpave gyratory compactor
- Review the Superpave mixture requirements
- Summarize the moisture sensitivity test



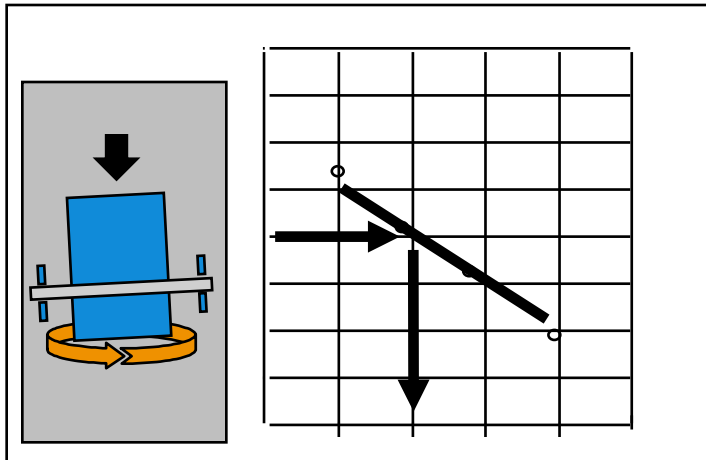
# Four Steps of Superpave Mix Design



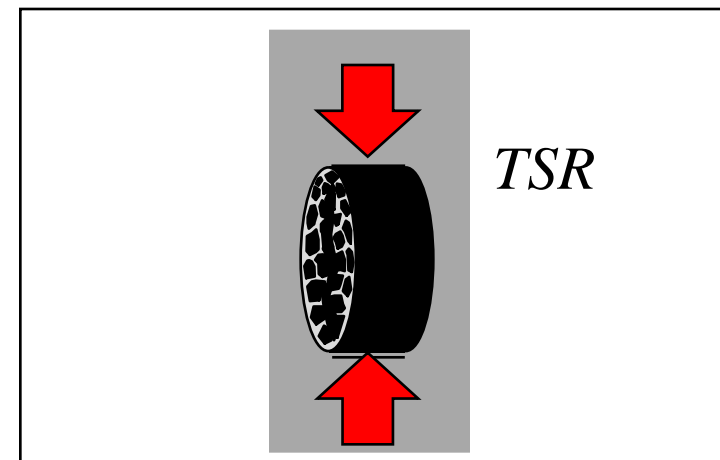
1. Materials Selection



2. Design Aggregate Structure



3. Design Binder Content



4. Moisture Sensitivity

# Goals of Compaction

Simulate field densification

Traffic

Climate

Accommodate large aggregates

Measure of compactability

Conducive to QC



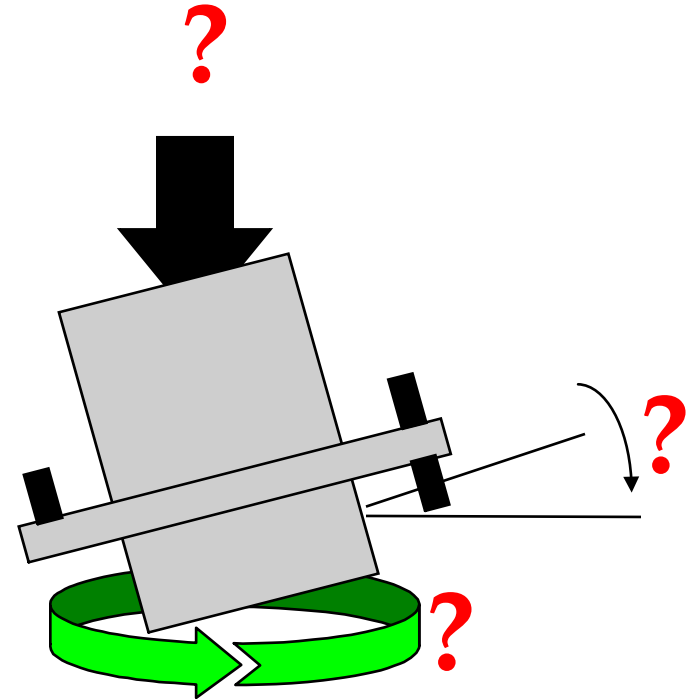
# Superpave Gyrotory Compactor (SGC)

## Basis

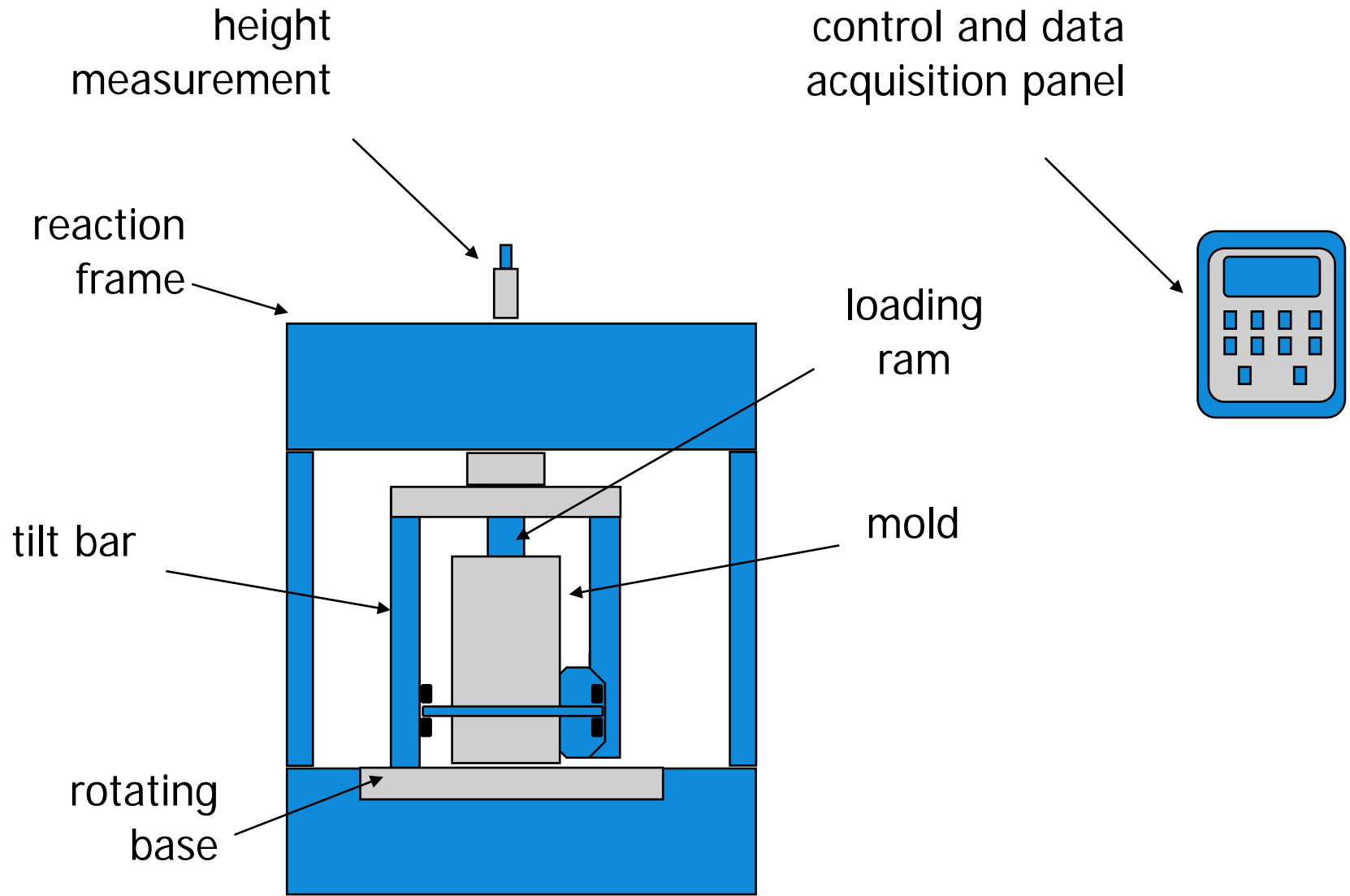
Texas equipment  
French operational  
characteristics

150 mm diameter

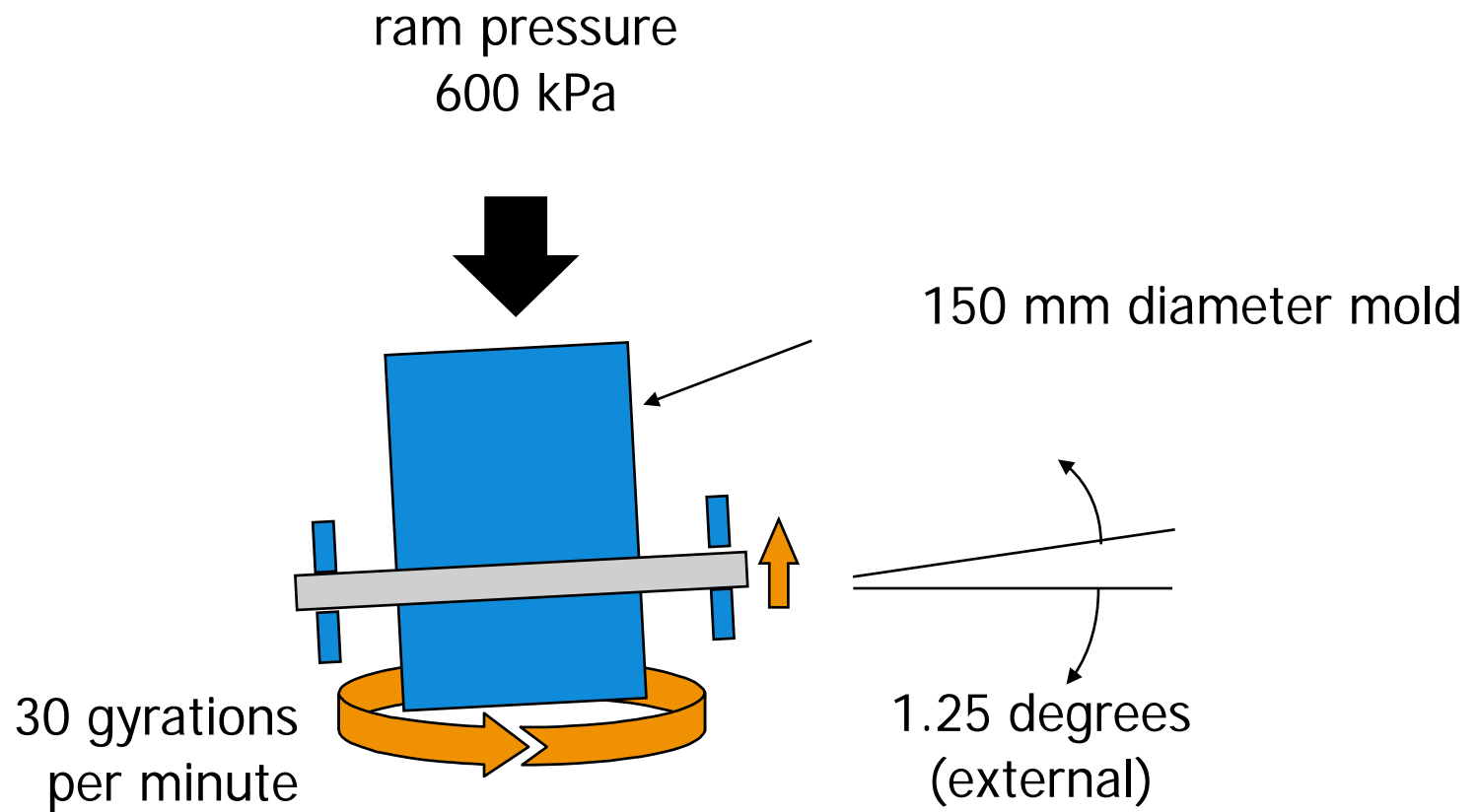
Up to 37.5 mm nominal size  
Height recordation



# Superpave Gyratory Compactor (SGC)



# Superpave Gyratory Compactor (SGC)



# Superpave Gyrotory Compactor (SGC)



# Specimen Preparation

Mechanical mixer

0.170 Pa-s binder viscosity

Short term oven aging

4 hours at 135° C

2 hours at Compaction  
Temperature (optional)



## Specimen height

Mix design -  $115 \pm 5$  mm (4700 g)

Moisture sens. - 95 mm (3500 g)

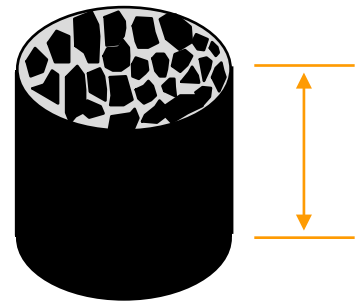
## Loose specimen for max. theor. (Rice)

Varies with nominal max size

19 mm (2000 g)

12.5 mm (1500 g)

**150 mm**





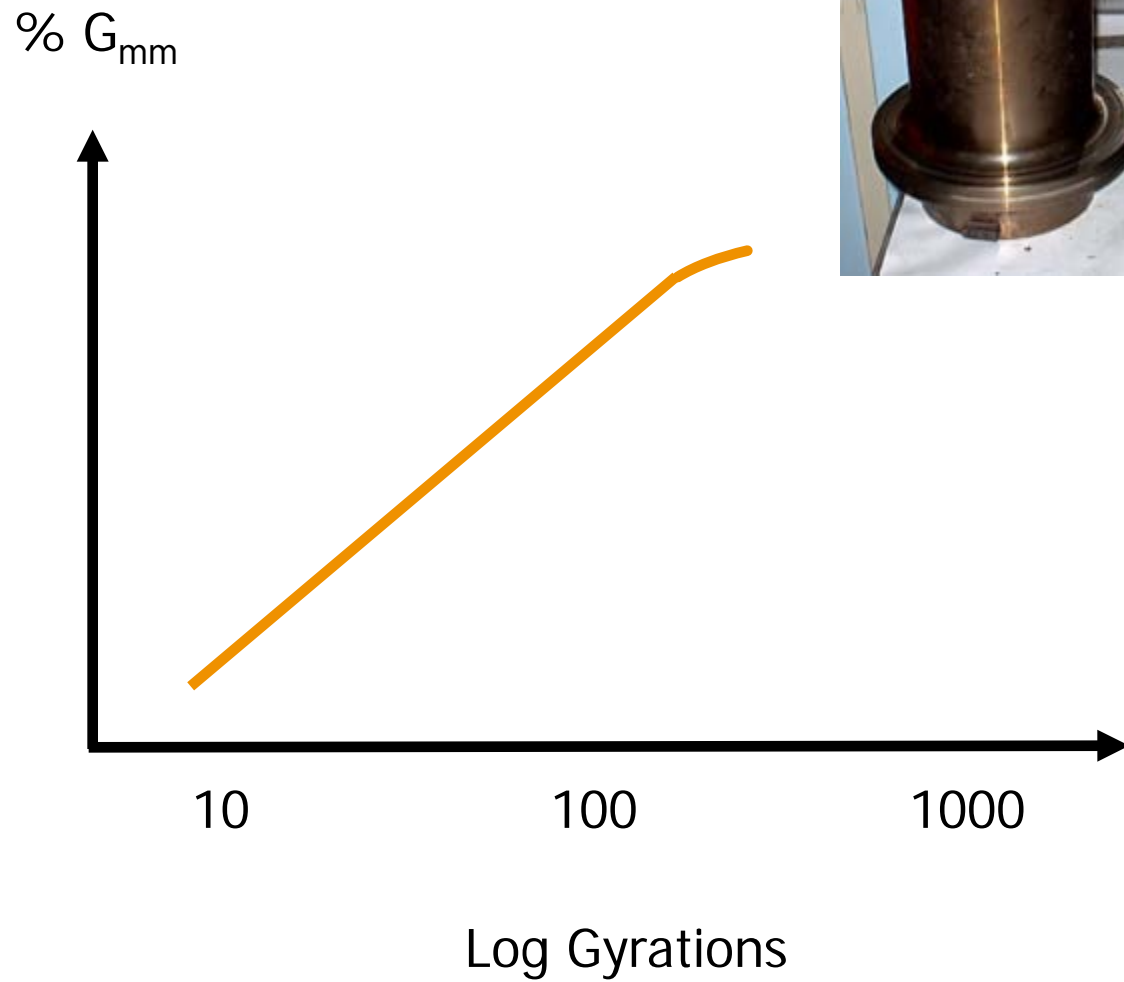
# Specimen Preparation



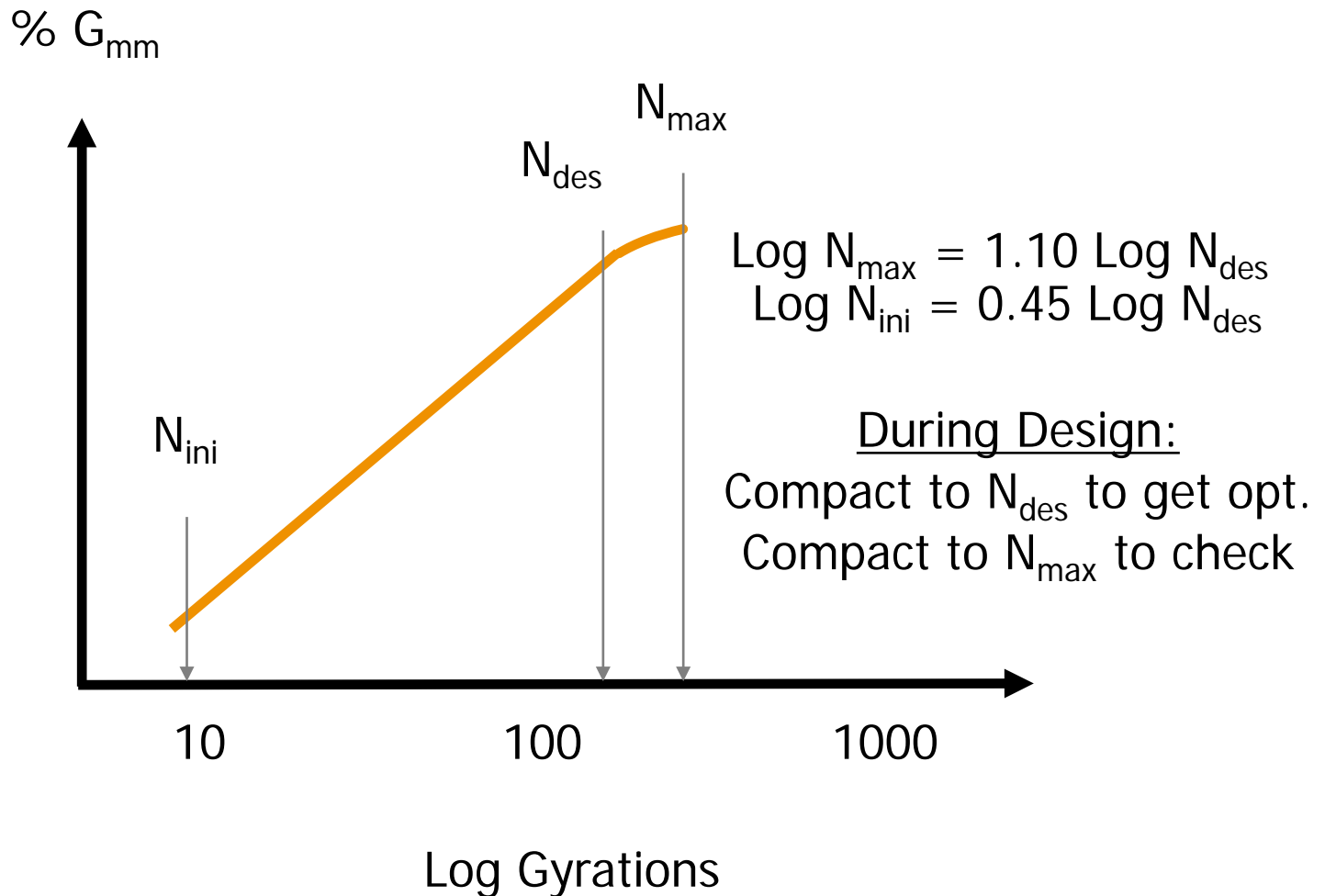
# Specimen Preparation



# Three Points on SGC Curve



# Three Points on SGC Curve



- $N_i$ : measure of mixture compactability during compaction
- $N_{des}$ : expected density in the field after the indicated amount of traffic
- $N_{max}$ : laboratory density that should never be exceeded in the field

# $N_{ini}$ , $N_{des}$ , and $N_{max}$

Traffic Level	Compaction Level		
	N initial	N design	N maximum
< 0.3	6	50	75
0.3 to < 3.0	7	75	115
3.0 to 30.0	8	100	160
> 30.00	9	125	205



# Superpave Mixture Requirements

## Mixture Volumetrics

Air Voids ( $V_a$ )

Voids in the Mineral Aggregate (VMA)

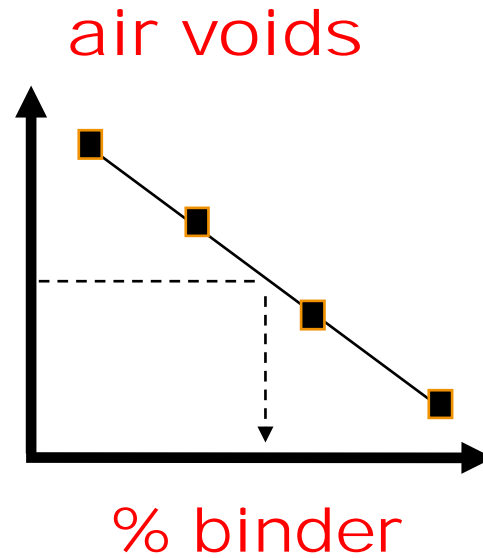
Voids Filled with Asphalt (VFA)

Mixture Density Characteristics

## Dust Proportion

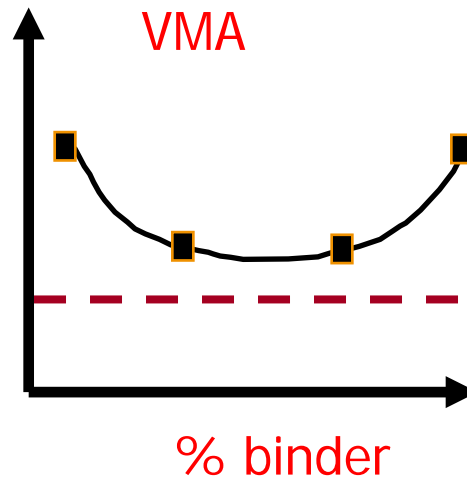
## Moisture Sensitivity

# Mix Air Voids Requirement



4 % at  $N_{des}$  Regardless of  
the Traffic Level

# VMA Requirements



Nom Max Size  
(mm)

Minimum VMA  
%

9.5

15.0

12.5

14.0

19

13.0

25

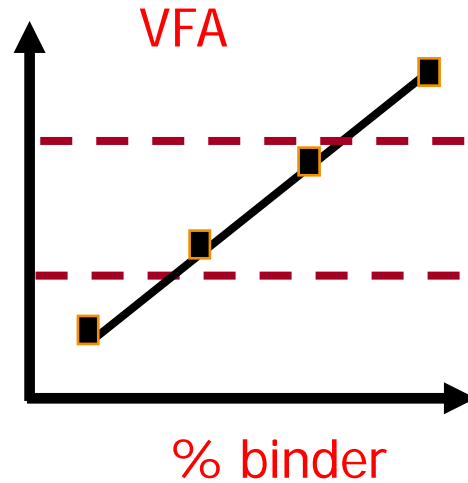
12.0

37.5

11.0

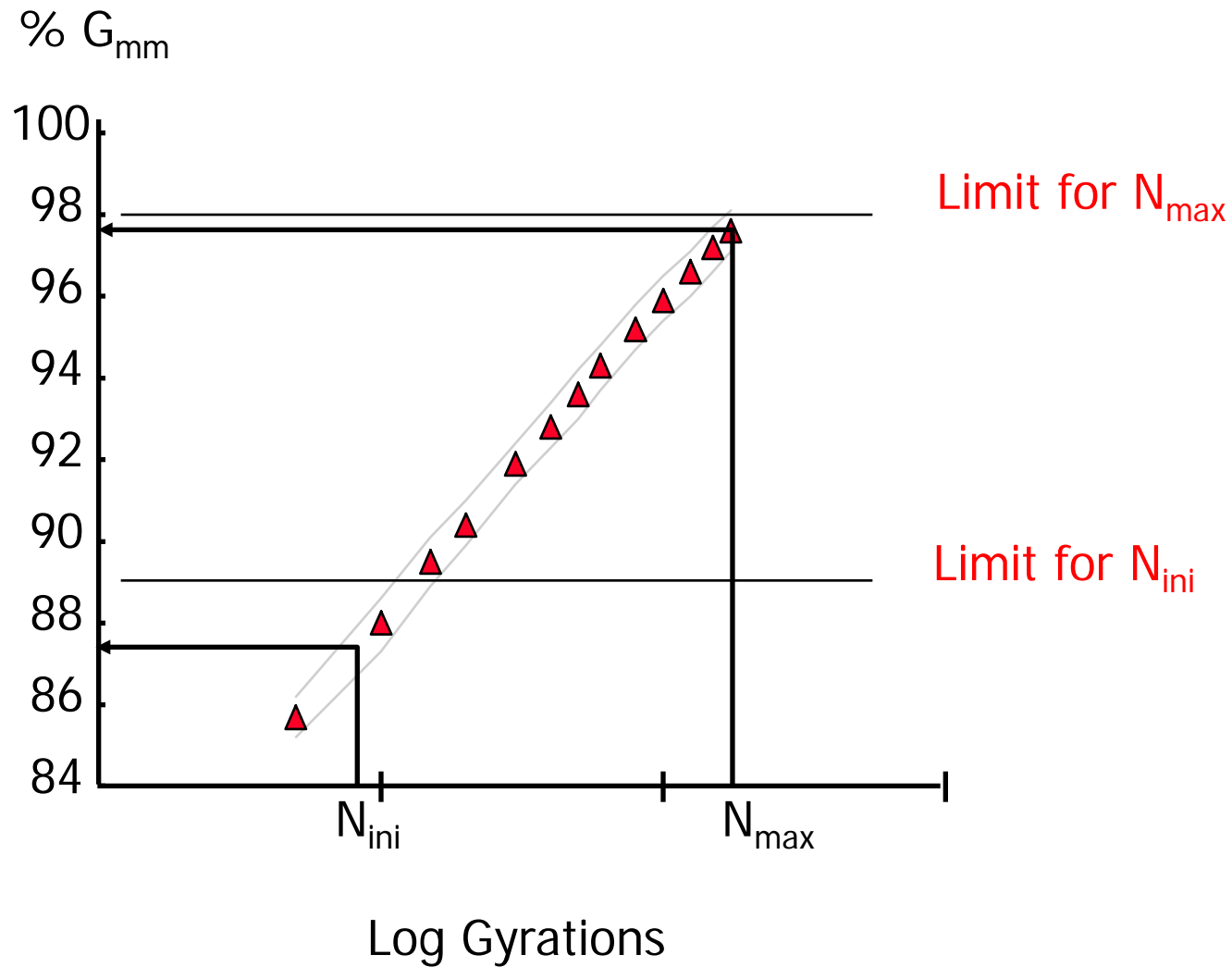


# VFA Requirements

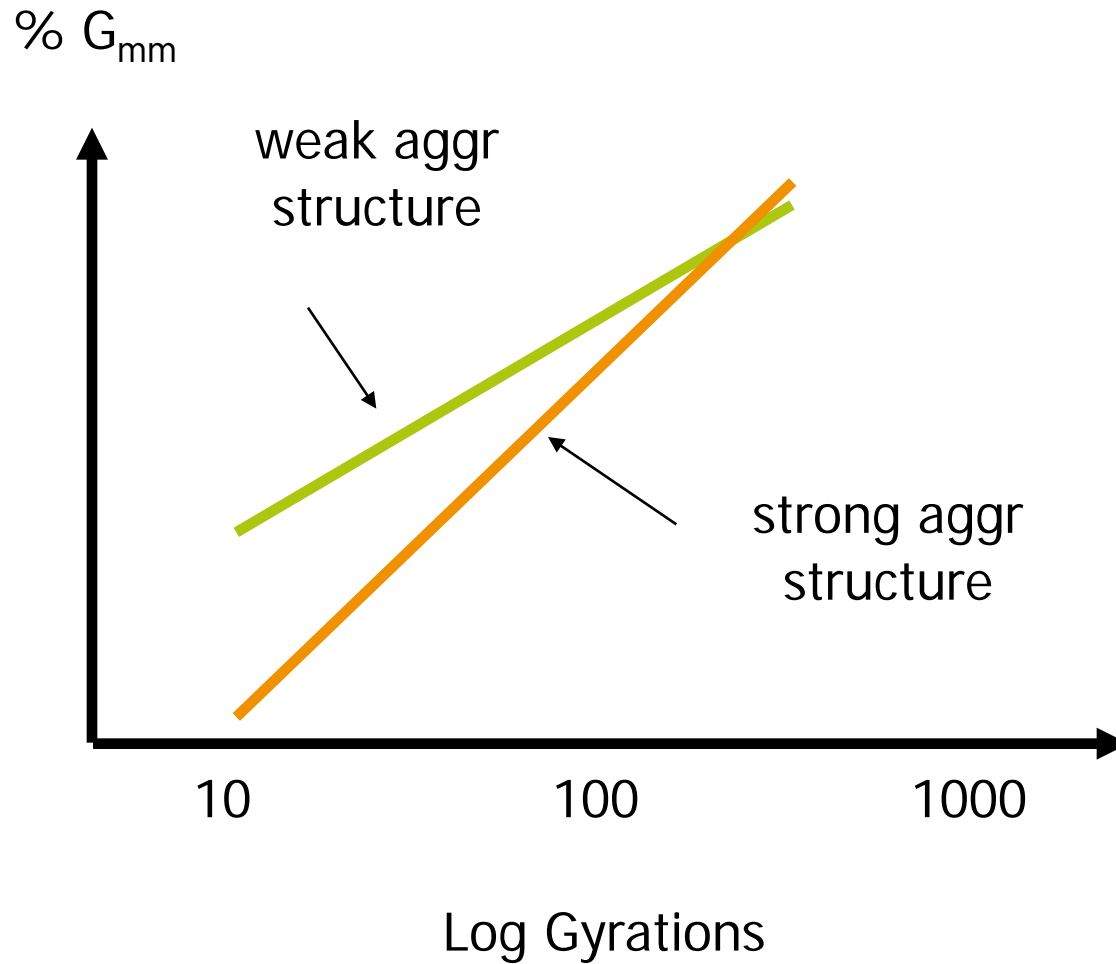


Traffic 10 <sup>6</sup> ESALs	Range of VFA %
< 0.3	70 – 80
0.3 to < 3	65 – 7
3.0 to < 30	65 – 75
> 30	65 - 75

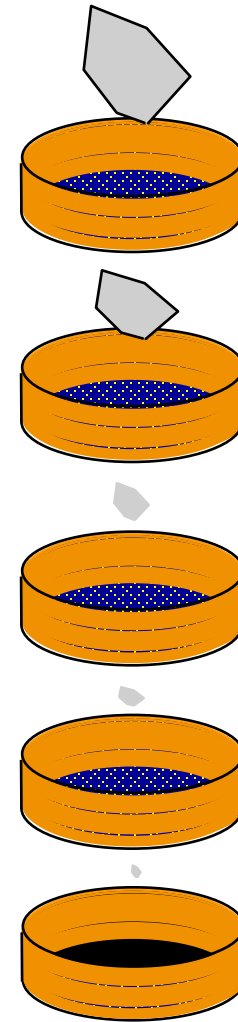
# Mixture Density



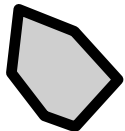
# Evaluate Aggregate Structure



$$0.6 \leq \frac{\% \text{ weight of } - 0.075 \text{ material}}{\% \text{ weight of } \textit{effective} \text{ asphalt}} \leq 1.2$$



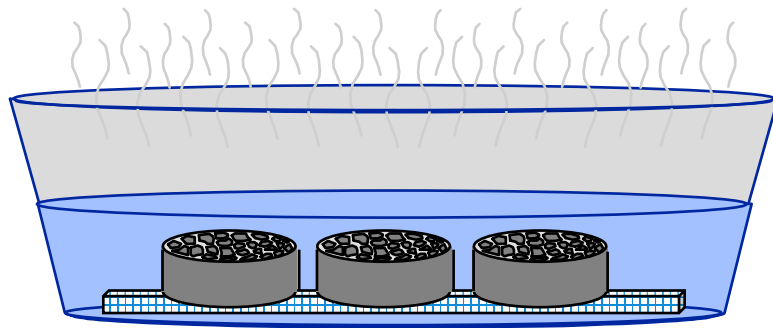
100
100
92
83
65
48
36
22
15
9
4



**Unabsorbed binder in mix**

# Moisture Sensitivity AASHTO T 283

Measured on proposed aggregate blend and asphalt content



**3 Conditioned Specimens**



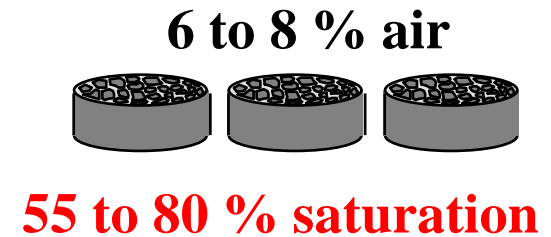
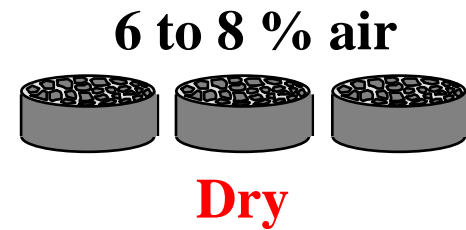
**3 Dry Specimens**



**80 %  
minimum**

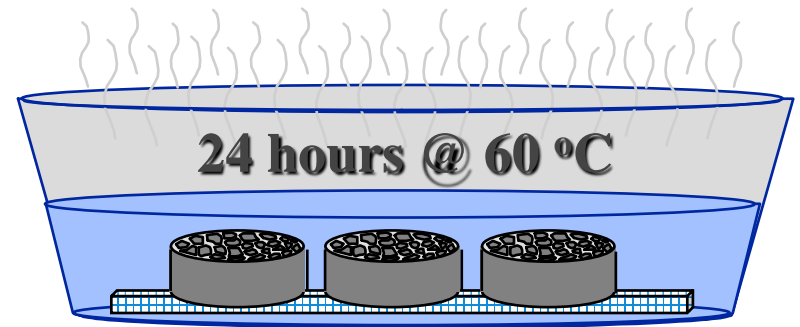
**Tensile Strength Ratio**

- Short term aging
  - Loose mix 16 hrs @ 60° C
  - Comp mix 72-96 hrs @ 25° C
- Two subsets with equal voids
  - One “dry”
  - One saturated

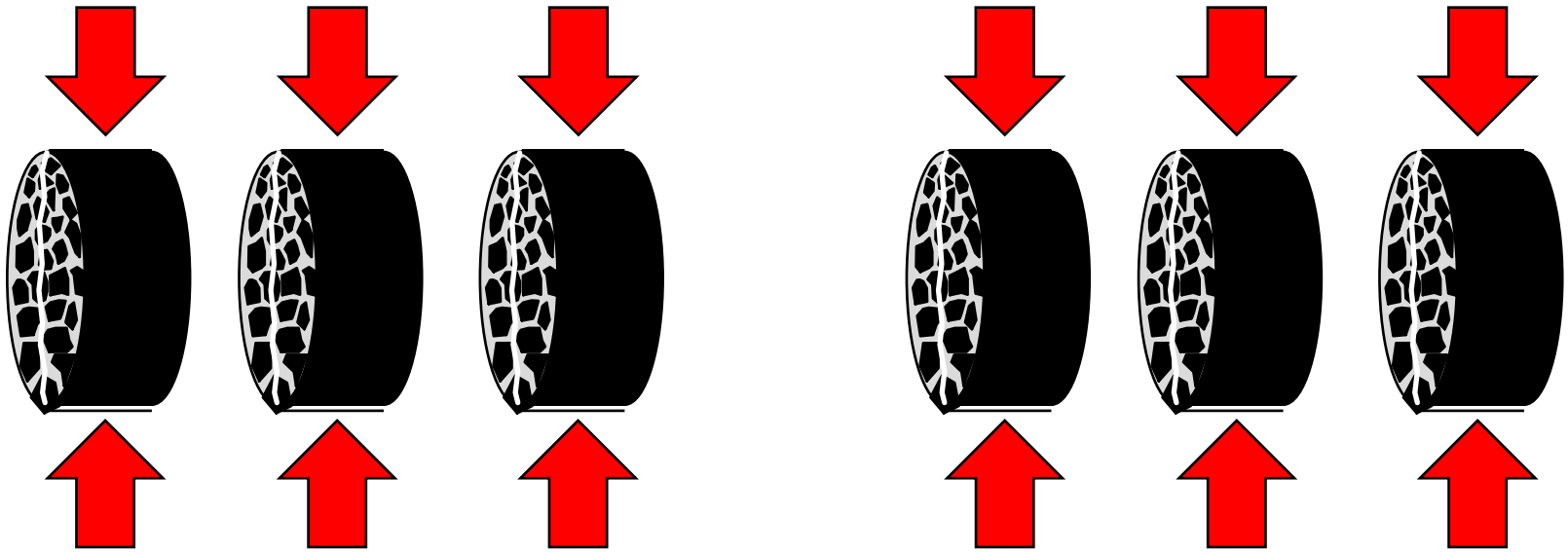


# AASHTO T 283 Conditioning

Optional freeze cycle  
Hot water soak



*51 mm / min @ 25 °C*



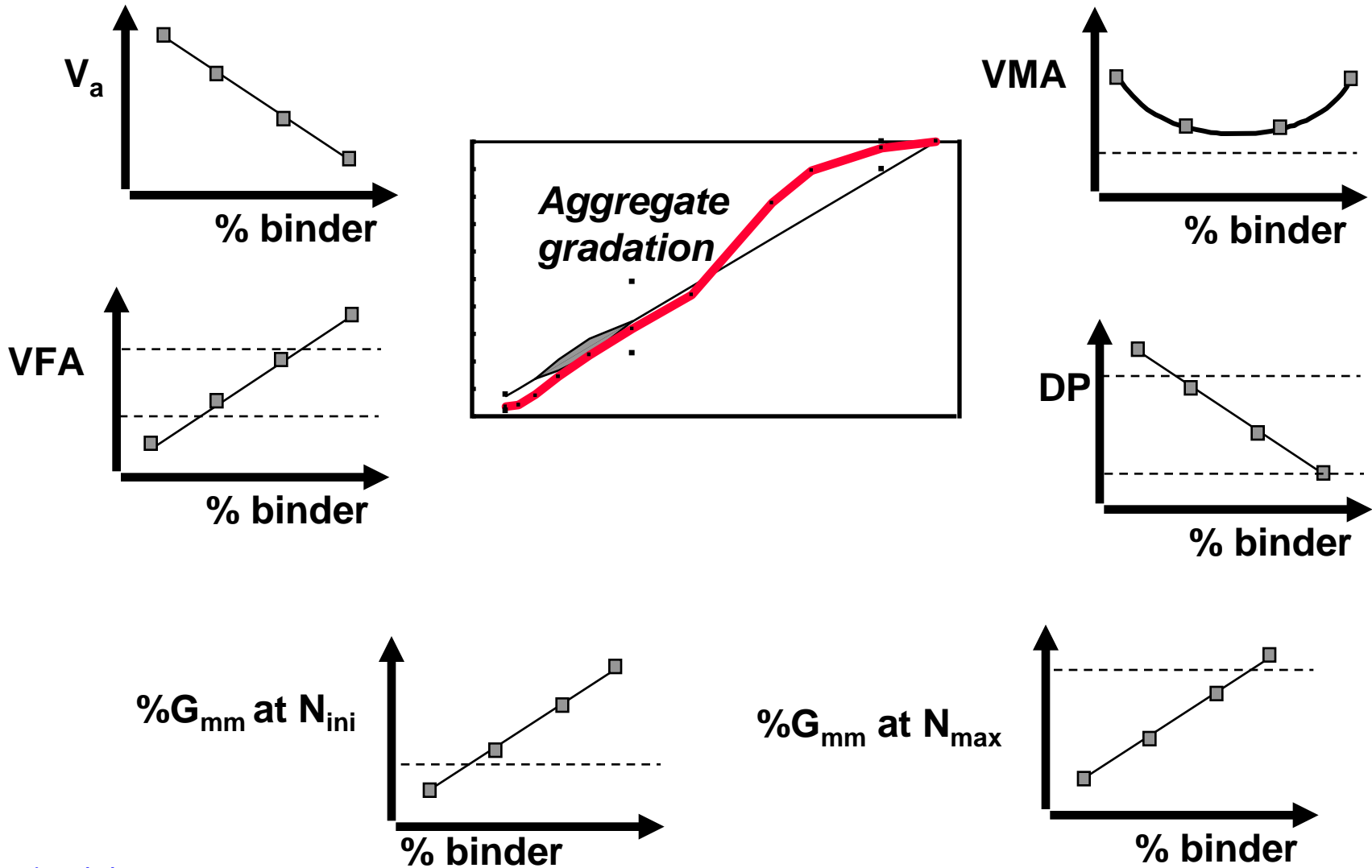
**Avg Dry Tensile Strength**

**Avg Wet Tensile Strength**

$$\text{TSR} = \frac{\text{Wet}}{\text{Dry}} \geq 80 \%$$



# Selection of Design Asphalt Binder Content



# Look for the Unusual!!!

