

Paper Presentation and Peer-Review
Group 1



ROYAL INSTITUTE
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Freeze – Thaw Performance Assessment of Stabilized Pavement Foundations

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Freeze and thaw phenomenon

- ❑ Temperature decreases: water freezes
- ❑ Volume increases
- ❑ Temperature increases: ice melts
- ❑ Saturated soil: weakening of the support capacity
- AASHTO 1993 design guide: effective roadbed soil resilient modulus base on the month of the year
- The Mechanistic-Empirical Pavement Design Guide (MEPDG): equivalent subgrade resilient modulus based on:
 - Regional climatic data
 - Time after thawing
 - Soil properties





Site conditions

- Project constructed in Boone, Iowa in May-July 2012
- 16 different cross sections
- Length from 674 ft to 1348 ft (205 m to 410 m)
- Test done in October 2012 and April 2013 (during one season of spring-thaw)

Test Section Description	Foundation Layer Profile ^a (above natural subgrade)	
Control	6 in. CLS ^b	12 in. compacted subgrade ^c
Mechanical Stabilization	6 in. CLS ^b	12 in. mechanically stabilized subgrade
4 in. Geocell + NW Geotextile	2 in. CLS ^b	4 in. geocell reinforced CLS, NW geotextile
6 in. Geocell + NW Geotextile	1 in. CLS ^b	6 in. geocell reinforced CLS, NW geotextile
NW Geotextile	6 in. CLS ^b	NW geotextile
Woven Geotextile	6 in. CLS ^b	woven geotextile
BX Polymer Grid	6 in. CLS ^b	biaxial geogrid
TX Polymer Grid	6 in. CLS ^b	triaxial geogrid
MP Fibers + PC Subbase	6 in. CLS ^b	6 in. recycled subbase + 5% PC + 0.4% MP fibers
FP Fibers + PC Subbase	6 in. CLS ^b	6 in. recycled subbase + 5% PC + 0.4% FP fibers
PC Subbase	6 in. CLS ^b	6 in. recycled subbase + 5% PC
Recycled Subbase	6 in. CLS ^b	6 in. recycled subbase
PC Subgrade	6 in. CLS ^b	12 in. 10% PC stabilized subgrade
10% FA Subgrade	6 in. CLS ^b	12 in. 20% fly ash (Port Neal) stabilized subgrade
15% FA Subgrade	6 in. CLS ^b	12 in. 15% fly ash (Ames) stabilized subgrade
20% FA Subgrade	6 in. CLS ^b	12 in. 10% fly ash (Muscatine and Port Neal) stabilized subgrade

^a All thicknesses provided are nominal.

^b CLS = crushed limestone subbase GP-GM or A-1-a (7% fines content).

^c Existing subgrade scarified, moisture conditioned, and compacted.

In situ testing methods

➤ Falling Weight Deflectometer

- 11.81 in. (3.6m) loading plate
- Load varies from 5000 lb to 15000 lb (2200 kg to 6800 kg)
- Deflections recorded by seismometers



Above: Kuab FWD, used for this test

$$E_{FWD} = \frac{(1 - \eta^2) \cdot \sigma_0 \cdot r}{D_0} F$$

E_{FWD} = elastic modulus [psi]

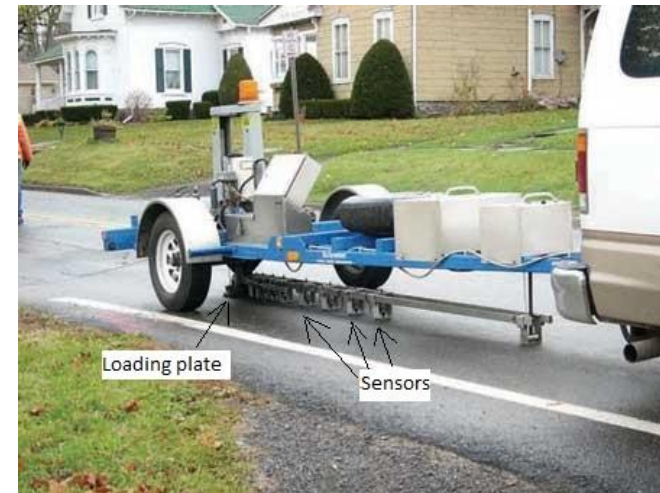
D_0 = vertical deflection [in.]

$\eta = 0.4$ = Poisson's ratio

σ_0 = applied stress [psi]

r = radius of the plate [in.]

$F = 2$ = shape factor



In situ testing methods

➤ Dynamic Cone Penetrometer

- Dropping of a 17.6 lb (~8 kg) from a height of 22.6 in.
- Measurements of the penetration
- Determination of CBR

$$CBR = \frac{292}{PI^{1.12}} \text{ for all soils with } CBR > 10$$

$$CBR = \frac{1}{(0.017019PI)^2} \text{ for subgrade } CBR < 10$$

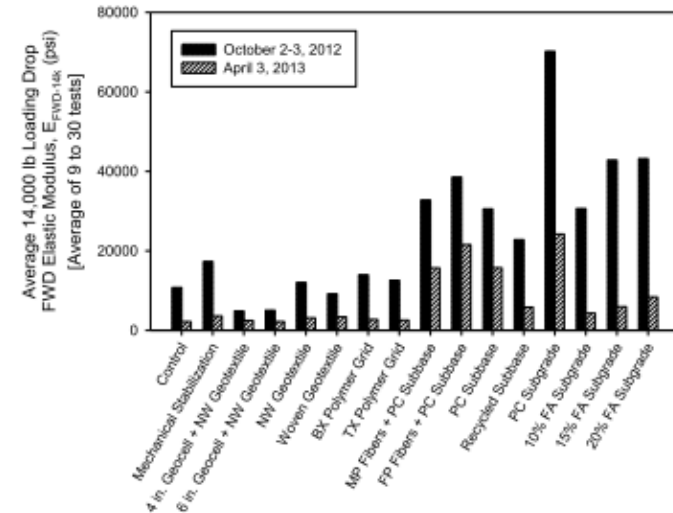
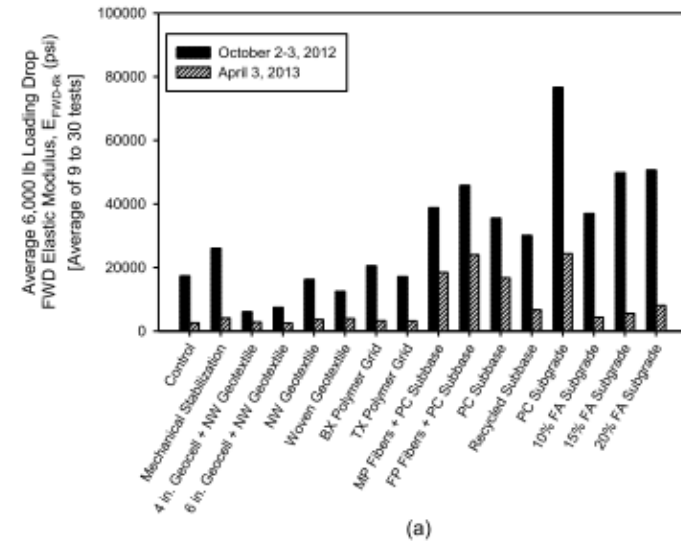
CBR = California Bearing Ratio

PI = Penetration Index $\left[\frac{mm}{blow} \right]$



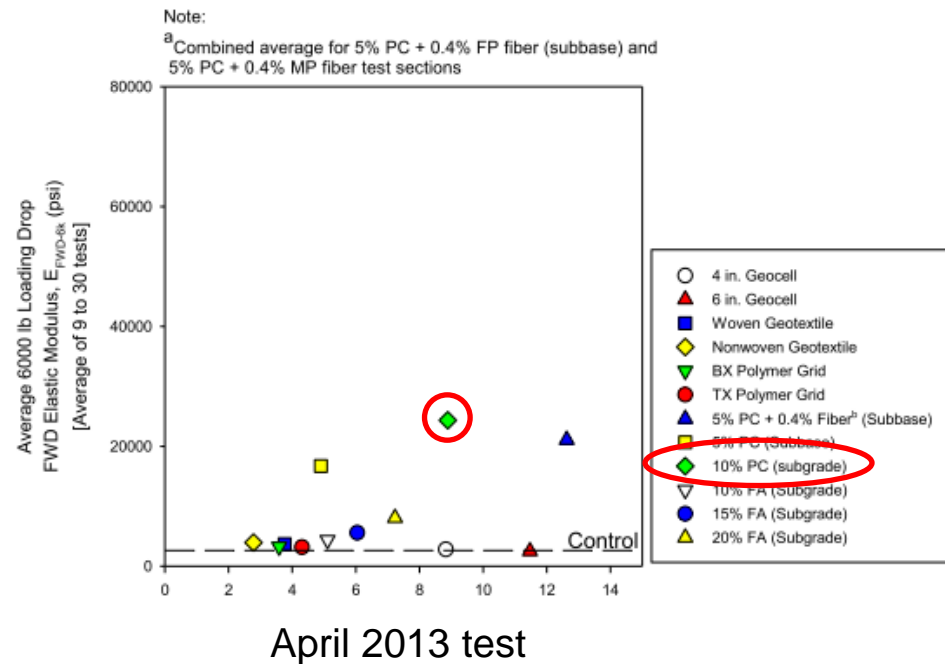
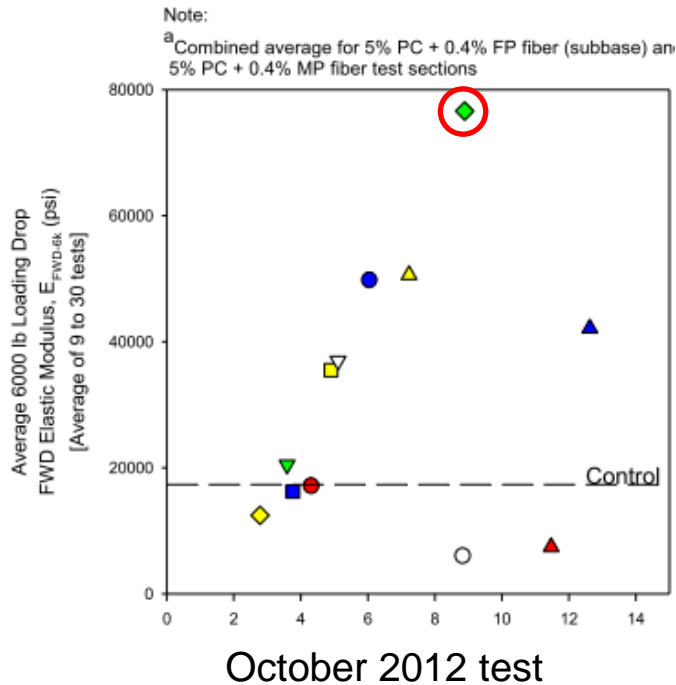
Results

- ❑ FWD results showed that all sections experienced a significant decrease in E_{FWD} . Cement stabilized shows the best behaviour, the same for DCP test.
- ❑ Statistical analysis showed correlation between the two tests.
- ❑ A cost analysis is carried out.



Cost Analysis

- Pavement foundation stabilization must be cost effective.
- 6 different contractors have submitted a cost analysis.
- Higher investments with the use of cement stabilization contribute to better pavement performance.





Review

- Positive aspects:
 - Good paper organization, the language is fluid and not hard to understand
 - The paper content reflects what the reader expects
 - Cost analysis, very useful and reliable.

- Negative aspects:
 - More pictures!
 - Problems with units
 - Statistics, not easy to understand!
 - Test made in USA, what about other countries?

Questions?

